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REPORT AND PRELIMINARY RESULTS OF POSEIDON CRUISE P386: NAIL (Nice Airport Landslide),

La Seyne sur Mer, 20.06.2009 – La Seyne sur Mer, 06.07.2009.





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Preface

The expedition P386 *NAIL* (Nice Airport Landslide) aims to shed light on the controls of slope failure and submarine landslide processes at the Ligurian Margin in the proximity of the Nice international airport, southern France. The cruise was a follow-up project of two earlier expeditions in the same area (Kopf et al., 2008; Sultan et al., 2008) and put special focus on deepening the knowledge regarding the causes of the 1979 Nice airport landslide and tsunami, acquire crucial geological data and materials for the recently submitted IODP (Integrated Ocean Drilling Program) drilling proposal 748-full (Stegmann et al., 2009), and test state-of-the-art technology for long-term monitoring and data transmission. The latter is loosely related to the EU real-time network of excellence *ESONET* (European Seafloor Observatory Network).

Among the methods utilised during *Poseidon* Leg P386 were echosounding to complement existing bathymetric charts, *in situ* measurements to characterize the natural state of the potentially metastable slope, seafloor sampling and place long-term instruments to study sedimentological phenomena and further evaluate the performance of acoustic dat atransmission systems. The regional goals were restricted to a small area south of Nice as well as the adjacent Var Canyon in the Baye des Anges, where mass wasting at various scales is observed. The research during and after cruise P386 serves to test key hypotheses concerning the trigger mechanisms of the mass wasting, which include ground water charging of slope sediments, high excess pore pressures, creep and plastic shear in sensitive clay horizons, vertical loading owing to anthropogenic construction, and regional seismicity.

The majority of the efforts during P386 were dedicated to the failed shallow slope near the Nice airport in water depths between 15 to 50 mbsl (meters below sea level). In addition, the moderately deep Var Canyon was instrumented to quantify the amount of material ending up in this system once sediment gets entrained by the river Var or remobilised from the shallow slope. Finally, one deep-water location further southeast was revisited for the long-term test of acoustic data communication devices.

Logistically, cruise P386 was split into two halves because of the number and size of seagoing equipment (4 moorings, large Borel buoy, 2 long-term piezometers). In addition, a second vessel (*Poseidon III*) operated in the same study area for part of Leg B; it hosted research scuba divers and devices to study groundwater seepage along the slope.

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Abstract

Cruise P386 "NAIL" with R/V *Poseidon* studied the western Ligurian Margin off Southern France, and area in the northeastern part of the western Mediterranean Sea characterized by its active tectonism and frequent mass wasting. The region near the Var estuary close to the city of Nice is particularly suited for landslide research because it represents a natural laboratority where it is possible to study a series of trigger processes of geological and anthropogenic origin. The aim of this MARUM expedition was to:

- i. Study the Nice airport landslide and adjacent stable slope in 15-50 m water depth;
- ii. Deploy a number of mooring stations to study sedimentological processes in the deeper slope (200-1500 m water depth) in the Var Canyon; and
- iii. Set up a long-term seafloor unit and adjacent buoy to test the performance of data communication systems for marine applications such as observatories.

Accordingly, the wealth of initial results include the successful deployment of several moorings as well as the communication system with the buoy. Data will be retrieved once the instruments get recovered in early 2010, so no scientific conclusions can be drawn from these portions of the cruise. In contrast, a large number of results is already available from the sampling and measurements south of the airport of Nice. From the gravity cores taken in or adjacent to the headwall of the landslide, the majority showed extremely freshened pore water composition (usually in 0.6-3.4 m depths) related to groundwater charging at deep levels. Although ROV surveying in this area showed pockmark-type seafloor roughness, bottom water sampling by research scuba divers failed to attest freshened water at the seafloor. Cores further showed evidence for catastrophic emplacement of sediment packages which were tentatively related to the 1979 slide/tsunami event. In several cores, steep normal faults with mm-displacement further support active deformation in the recent past. Results from 65 cone penetration tests (CPT) showed variable penetration depth (max. 3.5 m sub-seafloor) and oftentimes excess pore pressure increase during the 30-60 mins. "dissipation" period.

In addition to the above, gravity coring as well as CPTesting was undertaken at all 9 positions proposed for mission-specific drilling (IODP proposal 748-full). Also, 11 cores were taken but left unsplit because of the necessity for post-cruise geotechnical testing. Those cores originate both from the Nice Airport landslide and its vicinity, but also from the deeper landslide complexes sampled in 2007 during cruise M73/1.

2. Introduction

Based on the fact that 60% of the Earth's population live within the frontal 50 km of the coast, considerable scientific and economic efforts are undertaken to shed light on the processes shaping ocean margins. One of the most prominent of these phenomena are submarine landslides, which often coincide with earthquake activity and other geohazards. Given the highly dynamic setting and complexity of collision zones in the Mediterranean Sea, many processes are still poorly understood. Among the shortcomings in understanding collision zones, the temporal variation of deep-seated processes as well as their manifestations at shallower levels is an emerging key question. As a consequence, scientific research has to focus on long-term measurements of key physical parameters that drive landsliding. In Europe, the EIU Network of Excellence *ESONET* has identified a total of 203 data end-users in 11 countries with a wide-spread need for data monitoring of the solid earth beneath the sea, the interface between the solid earth and sea, and the water column. The University of Bremen (Germany) and IFREMER Brest (France) have joined forces on a multi-disciplinary level to work at the Ligurian Margin within ESONET and the Integrated Ocean Drilling Program (IODP).

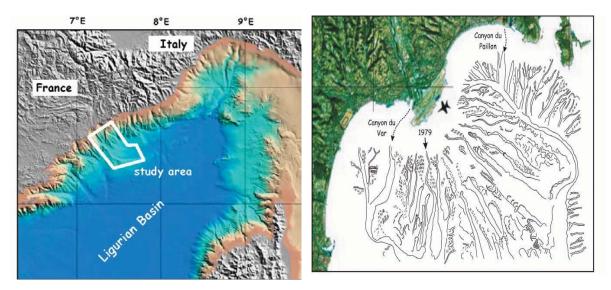


Fig. 1: (a) Bathymetric map of the Ligurian Sea and surrounding land masses; (b) Structural/geomorphic map of the study area at the Var estuary and canyon.

Several programmes and national as well as international initiatives dedicated considerable parts of their efforts towards submarine landslides. The 'The Deep-Sea Frontier' initiative (http://ec.europa.eu/research/environment/pdf/deepseefrontier.pdf), as well as its successor program DS³F ('Deep Sea & Sub-Seafloor Frontiers'; cf. Kopf, 2009)

by the EC have dedicated work packages concerning geohazards. Also, the IUGS-UNESCO IGCP511 project "Submarine Mass Movements and their Consequences" enters its 5th year with annual meetings and either books or special issues of journals resulting from the conferences (e.g. Solheim, 2006; Lykousis et al., 2007; Mosher et al., 2009). One of those held **ESF** workshop meetings, as Magellan Barcelona an (http://www.geohazards.no/IGCP511/; Camerlenghi et al., 2007), was co-sponsored by ECORD and set the spotlight on scientific ocean drilling. The proposal MEDSLIDES (Camerlenghi et al.; initially a pre-proposal to IODP in 2007, followed by a full proposal in 2008) was a major outcome here and spans the Eastern and Western Mediterranean including the Israel continental slope, Nile deep-sea fan, Gela Basin, Ebro Margin, Eivissa channel, and the Herodotus, Ionian and Balearic abyssal plains. In addition an IODPsponsored workshop on Geohazard drilling followed in Portland, Oregon in 2007 (see http://www.nsfmargins.org/Publications/Newsletters/Newsletter.html). A year later, another ESF Magellan workshop on "Drilling seismic hazards in European Geosystems" was held in Luleå, Sweden (Ask et al., 2008). Among other things, an APL concerning mass wasting events in the forearc to the Nankai Trough subduction system, Japan (proposal #738-APL; Strasser et al., 2008) and a mission-specific drilling proposal at the Nice slope (proposal #748-full; Stegmann et al., 2009) resulted as a direct consequence of that workshop.

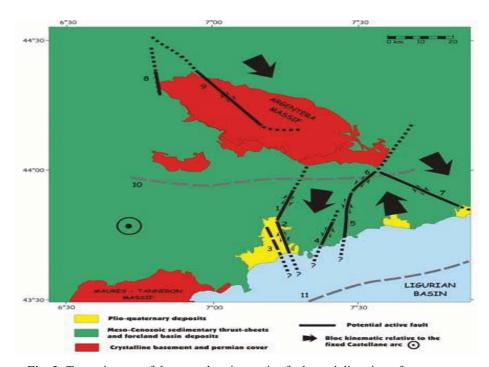


Fig. 2: Tectonic map of the area showing major faults and direction of movement. See text and cross section in Figure 4.

In general, the understanding of the temporal variations in geohazards is essential for many geoscientific research disciplines as well as mankind as a whole. Especially early warning networks for EQs and tsunamis have further become a major societal interest. The Ligurian margin is well suited for such a project because of its proximity, well known morphological and tectonic setting (Figs. 1-2), regional seismicity and landsliding (Figs. 3-4), and the wealth of existing data in the region. Cruise P386 aimed to broaden the data base in the area and install a number of long-term moorings to collect time series data offshore southern France.

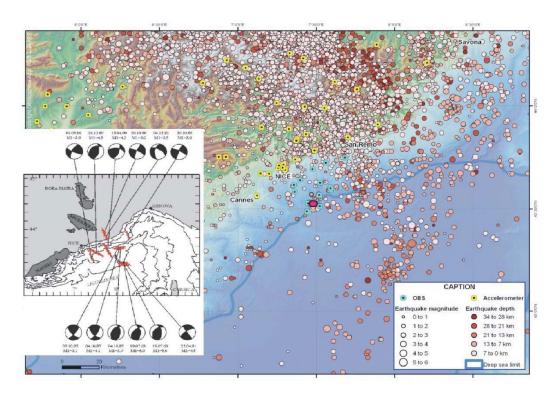


Fig. 3: History of seismic events in distribution in the southern Alps and Ligurian Sea, and focal mechanisms of Mw>3 earthquakes on the Ligurian Margin between 1960-2001 (from Henry et al., 2005, proposal 685-full).

3. Geological background

General Overview

The geological setting in the Ligurian Basin is that of a tectonically active and unstable margin. Much of the morphology on- and offshore as well as the main tectonic lineaments, their direction of movement and seismicity is illustrated in map view (Figs. 1-3) and SW-NE cross section (Fig. 4). Most of the sediment is received from the erosion of the Alps in the north (Fig. 1; see Mulder et al., 1998). Tectonic deformation rates are small, however, a background seismic activity is present, with the largest earthquakes occurring offshore (M~6 in 1887, and M 6.3 in 1963; see also Fig. 3). This activity may be explained by compression on the rim of the Alpine belt, which is now collapsing at least in the French-Italian part (Fig. 4). However, the active offshore fault network, consisting of both compressional and extensional structures, is incompletely understood (see Migeon et al., 2006).

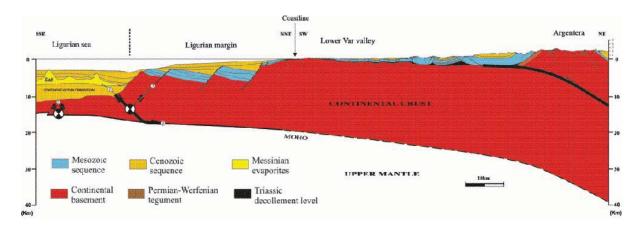


Fig. 4: Schematic cross section through the study area in SW-NE direction. Earthquakes are most abundant on the deep-seated crustal faults in the SW.

The Ligurian margin has a very narrow (or absent) continental shelf of ~2-3km width and a steep continental slope with a mean gradient of ca. 11° (Cochonat et al., 1993). The margin is fed with material from several small mountain-supplied rivers (Var, Paillon and Roya rivers) that experience semi-annual violent flash floods owing to snowmelt and convective rainfall (spring and fall). During floods, suspended sediment concentration can reach tens of kg/m³, resulting in hyperpycnal flows (Mulder et al., 1998). High and episodic sediment supply at the mouth of these rivers, cause deposition of thick under-consolidated deposits of variable grain size on the upper slope. In the Var canyon, the most relevant of the three river systems mentioned above, three types of seafloor failures are observed: (1) superficial slumping, which occurs in area of low slope angles in the upper slope, (2) Canyon-wall

gullying by undercutting currents and debris flows at the canyon wall, and (3) deep-seated failures characterised by pronounced headwalls at higher slope gradients and often unknown flow paths of the slid material (Klaucke and Cochonat, 1999; Klaucke et al., 2000). Adjacent to the Var estuary failures initiate on the upper slope, at less than 200 m of water depth, with scarps typically being approximately 100 m wide and mobilized volumes are less than 0,07 km³ (Migeon et al., 2006). Failure may either be catastrophic, or a successive strain accumulation from creeping via folding, slumping to mass wasting and deposition of debrites, turbidites, and landslide bodies.

The Nice Slope

The very narrow or absent continental margin off Nice (Fig. 5a), situated in the Baie des Anges and bounded by the prominent Cap d'Antibes Ridge (W) and Cap Ferrat Ridge (E), is characterized by a very steep slope characterized by deep erosion and canyoning with maximum slope angles of 27° along the side-walls of the canyons (Pautot 1981). The Var and Paillon canyons represent the most important erosive features, which are both linked to the fluvial systems, the latter being a negligible contributor of terrigenous supply at present (Klaucke et al., 2000). During the Messinian salinity crises, the Var paleo-canyon was shaped and filled during the Early Pliocene transgression and prograded in the Mid-Pliocene as a steep delta to the slope break, which corresponds to the modern coastline (Clauzon et al. 1990). Quaternary sedimentary sequences of the Var river mouth show a tripartite stack of facies (Dubar & Anthony 1995). The oldest Quaternary deposits are made of clast-supported gravel with a matrix of sand, silt and clay (Fig. 5b). A thick wedge of Holocene finegrained, shallow-marine and estuarine-deltaic sediments interbedded with river flood-plain paludal sediments is related to the postglacial marine transgression, whereas an upper fluvial channel gravel prograded a Gilbert-type fan delta recently. From a hydro-geological point of view these sedimentary sequences act as an aquifer system with pathways for conductive flow of meltwater or rainwater in the delta (see below). In the Var delta system, highly permeable strata (Pliocene substratum, Quaternary sandy gravel; see Fig. 5b) is confined by low-permeability muds (Anthony & Julian, 1997). Following their model the aquifer layer drains seaward at various levels down to a water depth of ~140m (Fig. 5b). Geochemical analyses in the upstream part of the Var (Guglielmi & Mudry, 1996), offshore in the water column overlying the Nice Slope (Guglielmi & Prieur, 1997), and piezometric measurements in the Nice slope sediment (Sultan et al. 2008) support the aquifer model and demonstrate a direct relationship between high discharge (flood) events (about 200 m³/s),

flow through the alluvial aquifer (Guglielmi & Mudry, 1996) and the occurrence of fresh water discharge into the sea (Guglielmi & Prieur, 1997). Regarding the superficial sediments of the uppermost Nice continental slope, major portions are unstable due to the underconsolidated state of the sediment owing to rapid deposition (Klaucke & Cochonat, 1999). This material has also been imaged during recent geophysical surveys (see Fig. 7 and Stegmann et al., 2009).

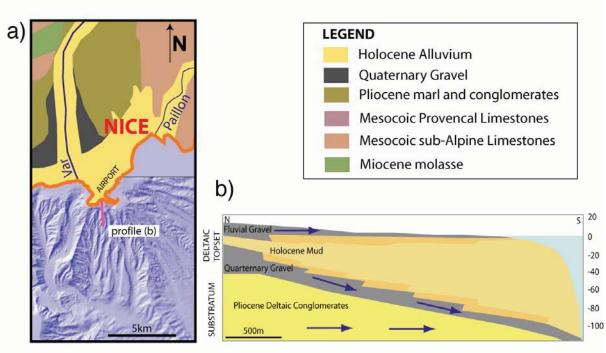


Fig. 5: Geology of the Nice area on-shore and the Nice slope off-shore compiled from previous workers (modified after Dubar & Anthony 1995 and Anthony & Julian 1997). (a) Map illustrating the different lithologies related to the evolution of the Var delta from the Pliocene to present; (b) schematic cross-section through the Var delta deposits in N-S-direction south of the Nice airport (see [a] for location).

The 1979 Nice Airport catastrophe

The abovementioned Nice Airport Landslide (*NAIL*, also the namesake of cruise P386) occurred on October 16th, 1979 on the Var prodelta of the Nice Slope (Figs. 5a, 6a). The NAIL area is surrounded by smooth, but narrow shelf with water depth ranging between 0m and 15m at the headwall. An embankment of the extended airport construction collapsed into the sea (Fig. 6a) and generated a tsunami wave of 2-3 m (Gennesseaux et al., 1980). Based on bathymetry data the volume of failed material was estimated ~8.7 x 10⁶ m³ (Assier-Rzadkiewicz et al., 2000), which was mobilized and transformed into a debris flow cutting two submarine cables tens of kilometres away from the sliding area (Hugot, 2000). The path of the failed mass is clearly expressed on the upper slope by a 4.5 km-long gully with depths between 25m and 40m (see Fig. 6a, arrows; Mulder et al. 1998). The airport was damaged (Fig. 6b), the tsunami affected the nearby village of Antibes (Fig. 6c), and the

former embankment was transported to more than 1000 m depth into the Var canyon (Fig. 6d).



Fig. 6: Location and transport pathway of the Nice Airport Landslide (NAIL) on Oct 16, 1979.

(a) The trace and velocity of the mobilised material running down the Nice Slope southward into the Var Canyon, and then redirected towards the West and then South again (data from Dan, 2007). Panels (b) and (c) are newspaper clipping from the Nice Matin the day after the event, illustrating the damage to the airport, here inspected by airport police (b) and the effect the tsunami had in the village of Antibes (c). In the course of detailed mapping of the seafloor in 1986, ROV dives discovered blocks having originated from the fill used for the failed harbor construction that got transported to the deeper part of the slope (d). Positions for panels b through d is given in Fig. (a).

Shortly after the accident, a detailed investigation of the bathymetry was started (Pautot, 1981) and several studies aimed to characterize the trigger mechanism(s). Reduction of sedimentary strength due to an earthquake can be excluded for this event, as no anomalous seismic signal was recorded. The MIP (MIP, 1981) proposed retrogressive failure, which initiated at the slope and then retrogressively reached the NAIL area. On the other hand, the tsunami wave following the slide lowered the sea level by ~2.5m, which resulted in static liquefaction of the overloaded slope (e.g. Seed, 1988). Both scenarios were tested by numerical modelling and the retrogressive failure mechanism was excluded as this kind of failure could not have provided the energy to generate the observed tsunami wave (Assier-Rzadkiewicz et al. 2000). Slope stability assessment under static conditions demonstrated a Factor of Safety (FS) >1., which in case of sealevel lowering of 2.5m decreases, but remains

>1 (Sultan et al., 2001). Unfortunately, the landfill operations preceding the airport extension, where 11 million tons of material at water depth of 25m and distances of up to 300m offshore had been additionally put on the slope six month prior to the failure (see details in De la Tullaye, 1989), were not considered in the study. Furthermore, the effect of overpressuring was disregarded, although the landslide occurred after several days of heavy rain (25cm in 4 days). Given that both the extra loading and episodic rainfall events (see http://www.hydro.eaudefrance.fr) remain crucial factors destabilizing the present-day slope, they were into consideration during recent studies (see Kopf et al., 2008; Sultan et al., 2008; Stegmann et al., 2009, below).

Previous work by participant group/institutions

Over the previous decade, the working groups at IFREMER Brest, CEREGE and MARUM Bremen contributed tremendously to the understanding of landslide processes in general (e.g. Sultan et al., 2004, 2007, 2008; Stegmann et al., 2007; Kopf et al., 2006, 2007, 2008, 2009), and the Ligurian Margin in particular. At the Nice slope, the most fundamental data set on is a hydrological model and set of questions and hypotheses is based upon is a grid of high-resolution MCS data with good penetration (>150 mbsf) acquired during M73/1 cruise in 2007 (see Fig. 7; and Kopf et al., 2008 for methodology and specifications). An example profile crossing the NAIL scar in N-S-direction is shown in Figure 7b to illustrate the overall geological situation and main lithological units.

The typical lithological succession on the Nice continental slope consists of three main units: (a) Along some profiles, lenses or homogeneous carpets of seismically transparent, ~10 m-thick "mobile layers" are found (Unit A). These have been identified earlier (Dan, 2007) and may represent modern spill-over deposits from snowmelt floods, recently mobilized material with a higher mobility (debris flow type or fluidized units). (b) Below the transparent series, parallel layered Quaternary deposits of variable thickness (a few m to ~50 m, Unit B) are probably Var delta sediments with various facies (clay, silt, sand and even gravel layers with components exceeding 10 cm in diameter; see cross-section in Fig. 5b). On the Nice shelf, those deposits are perfectly horizontal and in places show discordant contact to the inclined sand package. Closer to the shelf edge, prograding and aggrading delta deposits (i.e. topset to foreset beds) show tilting of the layers towards the Ligurian basin, possibly indicating deposition during a sealevel rise. Presumably, some of the uppermost layers may represent floodplain deposits, because the Var river mouth migrated westward over time (Sage, 1976). Inside the NAIL scar as well as in some locations down-

slope, the Holocene units lacks coherent internal layering and is hence difficult to interpret. While the surface topography may suggest displacement of some of these sequences (potentially from mass movement and sliding), other incoherent units lie topographically exposed and cannot result from downslope transport. (c) A S-dipping, well-stratified coherent package (Unit C) of sand/gravel or otherwise reflective, competent lithology, with its upper ~80 meters showing higher amplitude reflections (Fig. 7b). Its surface and contact to the overburden is predominantly smooth (in particular towards the deeper basin), but rough in places close to the coast. Nearshore, the top of the unit is interpreted as a former land surface during sea level low-stand. When compared to earlier work, this succession is presumably Upper Pliocene in age (see Savoye et al., 1993, their Fig. 12) and typically contains puddingstones, marls, sand and gravel (Guglielmi & Mudry, 1996). In a second seismic profile, the cascading normal faults in the mid-slope suggest that portions of the slope may have indeed been mobilised in a retrogressive manner (Fig. 7c).

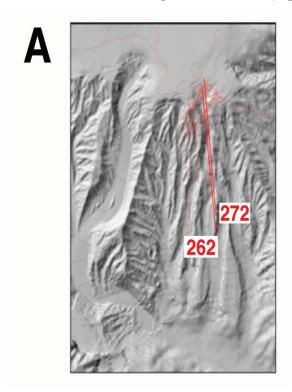
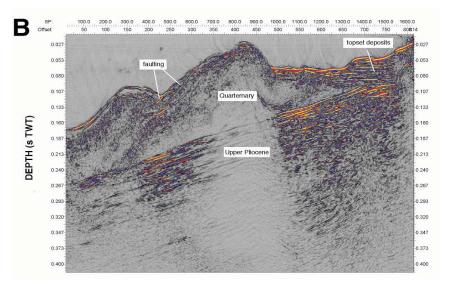


Fig. 7: (a) map with track chart offshore Nice from expedition M73/1 (Kopf et al., 2008), including N-S trending MCS lines 262 (b) and 272 (c) through the study area. Please note topset strata in line 262 (right) and possible evidence for retrogressive failure in line 272.



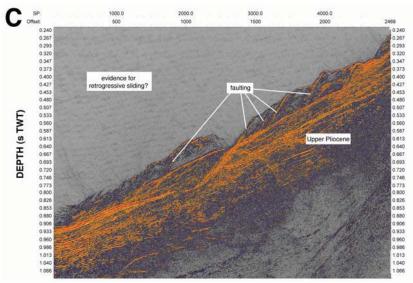


Fig. 7 continued.

In order to overcome limited penetration by gravity coring, pushed CPT profiling gathered *in situ* information on the strength of the sediments down to a penetration depth of 28m (Sultan et al., 2008). Those tests were carried out during PRISME cruise with R/V *L'Atalante* (fall 2007) using the Penfeld penetrometer (Meunier et al., 2004). The experiments focused on the stable western part of the Nice Slope. Based on 35 gravity cores (Fig. 8a) and 37 CPT profiles (12 pushed / 25 free fall) (Fig. 8b), which can also used to define the profiled sediments lithologically by soil types (e.g. Ramsey, 2002), significant sandy/silty layers were correlated successfully with commercial onshore CPT tests by SolsEssaies that were carried out shortly after the 1979 catastrophe. The coarser grained thin layers (cm scale) are deposited in the upper part of the slope in ~20mbsf and ~40mbsf (Fig. 8c). The *in situ* information also attests that (i) prominent sand/gravel layers are found in the Quaternary succession (Fig. 8c), (ii) developing shear zones in the Holocene foreset series,

as evidenced by low cone resistance and decreasing p-wave velocity when running the sonic CPT mode (see Fig. 9; Sultan et al., in press), and (iii) a sharp increase in cone resistance for 2 of the onshore tests that reached up to 60 m subbottom depth (Fig. 8c). This latter discontinuity is found in ca. 35-40 m depth, which corresponds to about the extrapolated top level of the Pliocene package (unit a; see above).

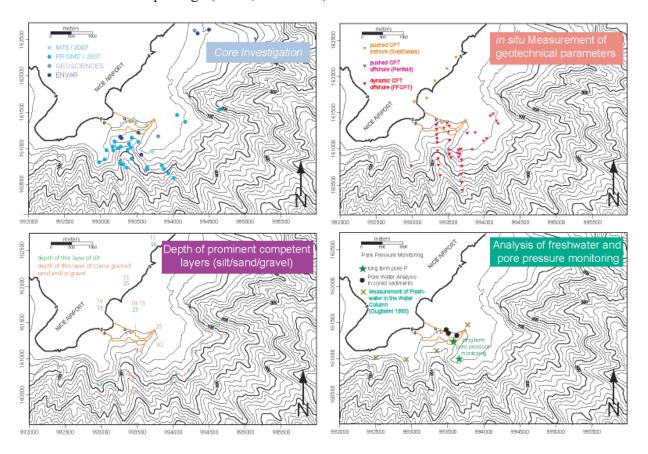


Fig. 8: Compilation of data collected in the NAIL area during several previous expeditions, showing locations of a) gravity cores, b) *in situ* measurements of sedimentary strength and pore pressure with different CPT devices, c) depth information of prominent silt, sand and gravel layers from coring and CPT testing, and d) locations of freshwater occurrence in the sediments or in the overlying water body (the latter from Guglielmi & Prieur 1997).

Based on a finite element model relying on geotechnical and sedimentological data, Dan et al. 2007 postulated a combination of trigger mechanisms including (i) the creeping of the sensitive clay layer, (ii) the point load of the construction and, (iii) the circulation of fresh water in the permeable sandy layers. Furthermore, their calculation attested the metastable state of the Nice Slope before and after the construction work. The "sensitive layer" hypothesis is supported by the good correlation between the maximum thickness of the sliding mass (max. 38 m) and the depth of the sensitive clay. Furthermore, the progressive failure scenario according to the creeping process agrees well with the observations mentioned in the official report (cracks, settlements, failures, collapses) following

landfilling operations (Seed et al., 1988). Due to the presence of these sensitive, mechanically weak clay layers, failure of the Nice Slope could have occurred regardless of the additional load posed upon the slope by the construction (Savoye et al., 2005; Dan et al., 2007). These zones exhibit a significant loss of strength between 3 and 9 mbsf during CPT profiling in the non-failed area (Sultan et al., in press). Following Leroueil et al. (2001) these zones can be defined as shear zones often associated with progressive failure (Thakur et al., 2006).

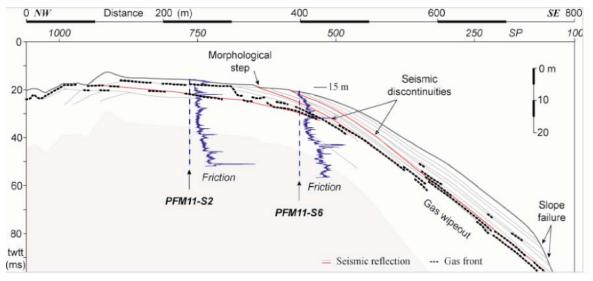


Fig. 9: Results from two Penfeld CPT deployments in the area immediately east of the NAIL scar. Data show low p-wave velocities (not shown here) and low frictional resistance in the slope apron where cklay minerals are abundant and minor amounts of gas are also suspected (from Sultan et al., in press).

Another crucial factor that episodically saps the stability of the Nice Slope is the hydrological regime in this area. For instance, the 1979 landslide occurred after a period of exceptionally heavy rainfall and river discharge (see above). Submarine freshwater sources are known to occur along the Ligurian coast (see Gugliemi, 1993; Guglielmi & Prieur, 1997; see above), which can affect the stability of the slope by lowering the effective strength of clays. Geochemical analyses on pore water extracted from coarse-grained, permeable layers in gravity cores attest the occurrence of fresh water in several locations within the NAIL scar (see Fig. 8d, and Kopf et al., 2009). Furthermore, a long-term piezometer installed in the NAIL scar recorded an increase in pore pressure linked to the amount of precipitation. Mid-term CPTU deployments in the same area have equally attested ambient overpressures up to 8 kPa, which in the location of deployment is very close to the overburden stress (Kopf et al., 2008).

Sedimentary processes in the Var canyon

Apart from landslide-related processes, the fate of material being mobilised in the shallow part of the Nice slope, the Var delta, or areas further upstream is of increasing interest. In general, canyon-turbidite systems collect, by a series of single, energetic and catastrophic events, a significant amount of the continental erosion products. Much of the material that accumulates at the shelf break or on the upper continental slope is in an instable situation and likely to move down the slope. Such movements impact on the slope and the deep-sea environments, determine the depositional architecture and evolution of deep-sea sedimentary systems. One of the distinctive feature of these turbiditic systems is the concentration and channelling of the terrigenous bed load from the mouth of large rivers (or the edges of platforms) to the abyssal plain. Processes involved in sediment transfers appear to be very efficient for particle grain size segregation and are a way to create huge sand accumulations in the deep-sea, providing in that way high quality reservoirs of high interest for the oil industry as it is moving deep offshore. These processes are also a major source of geohazards and damage for infrastructures lying on the sea floor and on coastal areas.

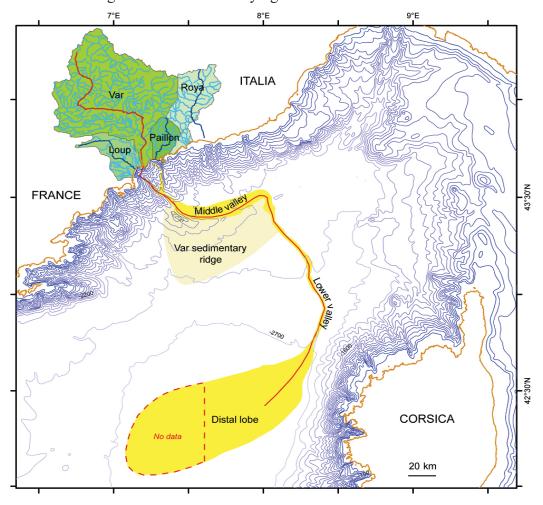


Fig. 10: The Var canyon-turbidite system from source to sink.

The sedimentary system of the Var is placed on an unusual passive margin, presenting a very narrow continental slope (2 to 3 km) and a steep continental slope that was reshaped during the Messinian crisis. The PlioQuaternary Var turbidite system has been supplied by 3 main canyons: the Var, the Paillon and Roya (Fig. 10). The canyons are active in both lowstand and highstand periods, with the preserved canyon activity during the present highstand period being a major research focus. The Var watershed is supposed to be the main sediment source of its fan (2830 km², highly mountainous). The Var river connects with the Var canyon head. The turbiditic activity in the canyon is hence controlled by the seasonal cycle of the river.

The mean flow of the Var river, at its mouth, is estimated to be around 53 m³.s⁻¹. Nevertheless the river regime is characterised by sudden floods surpassing 10 folds the mean runoff. The sediment discharge related to the Var river is not well estimated. Present days estimation indicates a total annual sediment discharge of 1,63.10⁶ tons/yr; extreme values being twice this estimation. There is significant evidence that during floods hyperpycnal flows occur at the head of the Var canyon. This regular and controlled activity is the main reason the Var turbiditic system may be considered as a natural laboratory for the study of turbidity currents.

Along the slope, the Var canyon presents a regular U shape during 25 km long. Its width increases from 300 m at the head to 1 km at its transition to the upper valley. Its depth presents a longitudinal variation as well. From 130 m depth at the head its incision reach 370 m further down the slope. The increase of the canyon transverse section correlates to the decreasing of its mean slope from 16 % to 3,3 %. Several terraces bordering the canyon present fine sediment deposits whereas the canyon is characterized by gravels and coarse sand. After its canyon, the Var system develops a long valley, 155 km long from canyon's confluence to the distal lobe at the base of the Corsican marge. The Upper Valley is 5 km long. Its width is comprised between 1,7 to 4,8 km and its slope varies from 3,3% to 3% at 2000 m water depth. The Middle Valley presents a long eastward turn of 50 km to 2500 m water depth where it turns again southwards. Its very wide channel (15 to 5 km) is bordered by a high and continuous right-hand levee, called the Var Sedimentary Ridge, above which turbiditic events regularly overspill. The left-hand levee is very low and discontinuous. At the end of the Middle Valley the slope has decreased to 0,4%. The 100 km long Lower Valley is characterized by a long, straight and shallow (20 to 30 m deep) by-pass channel and discontinuous flat levees. Its slope decreases to a very low value of 0,2% by 2700 m water depth at the location of the distal halocene lobe of 80 km long and 40 km wide.

The simplified morphology of the Var system and its present days activity constitutes an optimum to study and model turbidity currents. The direct connection to the Var river, the Canyon and the associated terraces, the overspilling ridge and the extent of its lower valley present singular features to constrain and be approached by numerical modelling.

4. Scientific rationale and State-of-the-art

(A. Kopf, P. Henry, S. Stegmann, J. Blandin, R. Silva Jacinto)

Overview

The stability of marine sediment at ocean margins is a function of the intrinsic strength of the material and forces counteracting this strength (e.g. Hampton & Lee, 1996). When broken down to the particle scale, the strength is controlled by the friction coefficient for the individual mineral particles at a given confining stress, minus the pore pressure that is compensating for some of the external stress. This relationship, known as the effective stress (Terzaghi, 1946), is a crucial aspect in slope stability since pore pressures may equal the overburden stress, exceed lithostatic values, and hence cause liquefaction (in coarse-grained sediment) or softening (in fine-grained material) by destroying the particle network (e.g. Maltman, 1994; Moore et al., 1995). Both non-destructive soft sediment deformation (creeping, slumping, liquefaction), as well as brittle failure (faulting, hydrofracture), are important processes in mass wasting along continental slopes.

The inherent mechanisms and factors governing slope stability and submarine landslides are known because of extensive research carried out by academia and industry (e.g. Hampton et al., 1996; Locat 2001; Locat & Lee, 2002; Mienert, 2004; Lee, 2009), however, the temporal and spatial variability of landslide processes remain poorly understood. In general, submarine landslides occur in areas of weakness, often posed by the presence of weak mineral phases such as clay minerals, or by excess fluid that enhances pore pressure. Spatially, slides are a global phenomenon occurring in fjords, river deltas and fan-canyon systems, open continental slopes and volcanic flanks (e.g. Huehnerbach & Masson 2004). Temporally, they are influenced by the sedimentology of depocenters as well as variations in seafloor pressure and temperature, seismicity and volcanic activity, or groundwater flow conditions (Lee, 2009). Although the abovementioned processes are broadly understood, the exact trigger mechanisms of only a few given submarine landslides are known with certainty (Mienert et al. 2003; Sultan et al. 2004). For the majority of slides, multiple triggers are usually considered, with pore pressure being the favourite, but ineluctably unsatisfactory explanation because of an uncompromising lack of evidence (see below).

Submarine landslides range greatly in their size, from small, frequently occurring failures in active environments such as coastal zones and canyons, to failures that involve hundreds of km³ of sediment but occur much more infrequently. In either case, they represent a major geohazard for offshore infrastructure (platforms, pipelines, cables and submarine

installations; e.g. Vaunat & Leroueil, 2002; Longva et al., 2003; Sultan et al., 2004) and, if tsunamigenic, to coastal structures and populations, both locally and in the far-field (Tappin et al., 2001). Slope failure is generally controlled by long-term governing factors and shortterm triggers (Leroueil et al., 2001; Locat 2001; Sultan et al., 2004). The first include topographic effects such as slope gradient, the geodynamic evolution of the margin (sedimentary or tectonic loading, unroofing, erosion, etc.) or other effects (glacial loading/unloading, marine transgression/regression, etc.; Lee, 2009). The second group of trigger mechanisms acts at a much shorter time-scale and usually causes a significant change in stress state. Among the processes most crucial to slope stability are (i) seismic loading (i.e. earthquakes), (ii) storm wave loading, (iii) rapid sedimentation (in deltas, through mass wasting, etc.), (iv) gas hydrate dissociation, (v) deep-seated fluid generation, upwardmigration and seepage, (vi) oversteepening, (vii) cyclic loading by tides, (viii) gas charging, and (ix) groundwater charging (see Locat and Lee, 2002 for details on many of those points). Despite the variety of processes in this list, one overarching aspect is the transient change in pore pressure that is the primary or indirect result of all of them. One of the key goals in landslide research is hence to establish the relationship between the preconditioning factors governing the area and the short-term triggers causing the slope to fail.

There are regions on Earth where wide stretches of ocean margins provide evidence for mass flows at many scales and a wide range of water depths (e.g. Maslin et al., 2004; Lee, 2009). Within Europe, the Mediterranean Sea represents such a region. Mass wasting has been reported from many of the large estuaries and delta systems in both the Eastern and Western Mediterranean (e.g. Ebro (e.g. Urgeles et al., 2006), Rhone, Var (e.g. Dan et al., 2007), and Nile fans, to name just a few), the seismogenic Algerian (e.g. Dan et al., 2009) or Ligurian margin (e.g. Klaucke & Cochonat, 1999), the Mediterranean Ridge and Cretan margin (e.g. Chronis et al., 2000), the Florence Rise, Anaximander mountains, Cyprean and Hellenic Arcs, or slopes of islands forming the Aegean volcanic arc. With approximately 46000 km of coastline, 160 Million along it (plus an additional 135 Million tourists each year, i.e., 30% of the global tourism), landslides pose a considerable risk. The French Riviera, northern Ligurian basin, seems particularly vulnerable because it is one of the areas with the highest population density (e.g. Marseille, Toulon, Cannes, Nice, Monte Carlo) and clearly with the largest economical vulnerability (i.e. insured capital). From these cities, Nice is the 5th largest city in France and the 2nd busiest international airport in France with a potential of 13 million passengers each year (Anthony, 2007).

When returning to tectonically active, hazard-prone zones like the Ligurian Margin, the key issue remains whether brittle failure in a formation or slip on an existing fault is triggered by fluid pressure transients, or solely by the mineralogy of the fault gouge (see competing hypotheses by e.g., Byerlee, 1978, 1990 vs. Rice, 1992). A considerable amount of work has been done in recent years to solve this problem (Logan & Rauenzahn, 1987; Saffer et al., 2001; Kopf & Brown, 2003), attesting that the pore pressure may be of similar importance as the friction coefficient of the gouge material itself. As a consequence, one of the central research goals into the behaviour and evolution of sediment failure is to separate the effects of intrinsic frictional properties and fluid pressure variations. For research expedition P386, the main aim is to clearly distinguish between those effects by

- Cone Penetrating Testing (CPT) where pore pressure and sediment strength are obtained,
- Measuring mid-term fluid pressure background and transients related to seismic events (in collaboration with IFREMER Brest, France, using mid- to long-term piezoprobes), and
- Doing selected laboratory tests on sediment gravity cores to measure the mechanical response to static and dynamic loads.

In situ geotechnical testing

Cone Penetration Tests are a widely used method for *in situ* sediment characterisation in onshore and offshore settings, both in science and industry (e.g., Lunne et al., 1997). With the autonomous, modular free-fall probes (hereafter as FF-CPT) developed in Bremen, a straightforward cost- and time-effective way was found to measure sediment resistance and pore pressure response to insertion into soft sediments. Measurements are carried out pogostyle, and after retrieval of the data disk, pore pressure evolution and sediment stiffness (measured as frictional resistance of the tip and a mantle sleeve; see Fig. 11a) as well as temperature are immediately available. In addition to the direct observations, other parameters can be rapidly estimated from the CPT data.

The maximum insertion pressure produced at the probe tip can be used to estimate the undrained shear strength of the sediment (C_u). For typical deep sea sediments, Esrig et al. (1977) suggest the relation: $C_u = U_{\text{imax}}/6$, where U_{imax} is the maximum insertion pressure. The decay of excess pore pressure produced by the insertion is governed by the consolidation process around the probe and can be modeled as radial consolidation. Bennett et al. (1985) predict the coefficient of horizontal consolidation (C_h) from the time taken for

50% of $U_{\rm imax}$ to dissipate, and C_h can then be used to determine the permeability k. In places where coring and Penfeld deployments are too time-consuming, pogo-style CPT measurements will not only provide first-hand *in situ* results, but with secondary parameters to estimate slope stability and hazard potential. In order to make use of the above *in situ* data in hazard mitigation, longer-term observations become a necessity. Temporal records of pore pressure changes have been demonstrated to correlate with regional tectonic stresses and seismic activity in a number of places (e.g. the Juan de Fuca Ridge; Becker & Davis, 2003; see also next chapter). For this reason, the previously used piezometer v1 as well as a refined design, the piezometer v2 (Fig. 11b), were to be deployed during cruise P386 for time series recordings of pore pressure and temperature in the Nice Airport slide scar and adjacent slope. This joint work by MARUM Bremen and IFREMER Brest represents the continuation of two successful cruises beforehand in the same area: R/V *Meteor* cruise LIMA-LAMO in July/August 2007 (Kopf et al., 2008) and R/V *L'Atalante* cruise PRISME in Oct./Nov. 2007 (Sultan et al., 2008).



Fig. 11: (a) MARUM CPT lance; (b) IFREMER piezometer v2; (c) MARUM dynamic triaxial apparatus; (d) MARUM ring shear apparatus.

Long-term pore pressure monitoring

Pore pressure is known not only for its prevalent role in faulting and other geodynamic processes (e.g. Rice, 1992), but also as a powerful proxy for strain (see Bredehoeft, 1967; Davis et al., 2004). So far, the routine procedure in ODP/IODP has been the deployment of tapered downhole probes (e.g. the DVTPP) during drilling, which have been used in a wide range of geological settings at shallow-moderate depths to obtain measurements of *in situ* pore pressure (e.g. Moore et al., 2001; Morris et al., 2003; Flemings et al., 2008). However, the interpretation of the probe measurements is often problematic, because of limited time

available for the induced pressure spike caused by probe insertion to dissipate. Another key element in ODP/IODP are pore pressure measurements in CORK long-term observatories (Circulation Obviation Retrofit Kit; cf. Davis & Becker [2001]) juxtaposing a cased borehole, which is hydraulically separated from the overlying water body. In the past, direct measurements of pore pressure have provided powerful constraints on regional scale flow models; for example, two CORK measurements at the Barbados margin (Foucher et al., 1997; Becker et al., 1997) within a few km of the trench provide tight bounds on the permeability of matrix and fault rocks in the outer wedge (Bekins & Screaton, 2007). In subduction zones, ambient pore pressure information (Davis et al., 2006) has been correlated with that inferred from laboratory consolidation tests on core samples collected during drilling (e.g. Karig, 1993; Morgan & Ask, 2004), and porosities obtained from shipboard index properties (e.g. Screaton et al., 2002) and inferred from seismic velocity (e.g. Cochrane et al., 1996; Hayward et al., 2003).

Using pore pressure signals as a proxy for seismic strain has been neatly demonstrated at the Juan de Fuca Ridge flank as well as the Nankai Trough accretionary complex (Davis et al., 2001; 2006). In the first case, a M 4.6 earthquake on the western flank of the Juan de Fuca Ridge caused a discrete pore pressure spike in three adjacent boreholes. The signal reached <0.2 kPa above background value some 70 km away from the epicenter, but up to 3.2 kPa excess pore pressure in 10-20 km distance at Site 1025 (for details, see Davis et al., 2001). Similarly, a series of low-frequency earthquakes were recorded in a CORKed hole (Site 808) in 2003, causing a 140 kPa pressure anomaly in some formations within the >900 m of instrumented hole through the frontal accretionary complex (Davis et al., 2006). Even seafloor seismometers and fluxmeters have been successfully used to suggest a relationship between seepage and seismic events in the vicinity (i.e., in the Costa Rica forearc, Brown et al., 2005). Along a similar line of methodology, IFREMER piezometers have been successfully recording pore pressure variations related to changes in charged aquifers in the Nice Airport Landslide scar for periods exceeding one year, with the excess signals reaching 15 kPa (Sultan et al., 2008).

Geotechnical laboratory experiments

A growing body of geotechnical laboratory work attests that frictional stability may be more important than the absolute shear strength of a material in controlling failure and sliding/faulting (see review by Scholz, 1998). Frictional stability is a function of the change in friction coefficient (µ) at a given effective stress, which is primarily rate dependent.

While high-porosity rocks show velocity strengthening (i.e. μ increases with increasing shear rate, e.g. during slip), low-porosity rocks weaken. In general, EQs are believed to nucleate in unstable materials, so that the replacement of clays in "weak" shear zones by precipitation (carbonate, zeolites, quartz) or mineral transformation processes would result in either unstable stick slip or conditionally stable behaviour (Moore & Saffer, 2001). Similarly, mechanically weak constituents such as sensitive clay minerals are believed to provide the failure plane for landslides and other hazardous mass wasting processes.

To ground-truth both our *in situ* data (see previous two paragraphs) and the geophysical information of the seafloor and shallow subbottom, designated soil mechanical laboratory experiments are part of the overall study P386 (see Ch. 6, *Methods* below). These include standard soil mechanical procedures to obtain the sediments index properties (wet and dry density, porosity), grain size distribution, liquid and plastic limits, and permeability and shear strength at various confining stresses and rates. Simple tests are conducted on board (vane shear apparatus, falling cone penetrometer) while more sophisticated ones will be carried out shore-based (e.g. dynamic triaxial tests [Fig. 11c] or ring shear tests [Fig. 11d]). Especially the latter suite of parameters provides a profound measure on slope stability on the seafloor, on how stable a given sediment may deform under certain conditions (namely at depth), and whether permeability allows for the build-up of significant pore pressures.

In addition, a number of locations from an earlier as well as a recently proposed IODP drilling expedition were proposed to be visited during P386 (Fig. 12). The main objective of this endeavour is to collect data for ground-truthing of some of the hypothesis put forward in the proposal (i.e. mechanical stability, pore water chemistry).

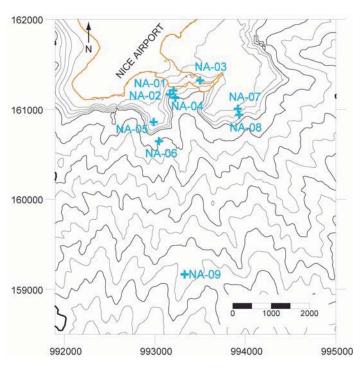


Fig. 12: Bathymetric chart of the NAIL area including the nine proposed locations for IODP mission-specific drilling (proposal 748-full; Stegmann et al., 2009).

Technological refinements for long-term measurements

Among the various types of sub-sea observatories, non-cabled infrastructures are worth considering given the following features that make them attractive in numerous scientific areas and situations:

- Their price is approximetaly 1 to 2 orders of magnitude less than cabled infrastructures, making them affordable to multidisciplinary teams of relatively modest size.
- They can be successively implemented at different sites within their lifetime, each time allowing the acquisition of long time series, without being necessarily left in place for more than 20 years.

They also represent a very useful way of investigating a site before making the decision of whether or not a heavier (and generally more pricy) cabled infrastructure is worth developing and deploying. However, the application field of non-cabled infrastructures is limited by the modest amount of data they can transmit. ADCPs, seismometers, echosounders, still and video cameras are among the sensors that are not impossible but still difficult to implement on non cabled observatories, when near real time communication of data is required.

Within the last ten years in Europe, several research institutions have implemented acoustic modems for observatory applications, often in close relationship with their manufacturer,

but the results were sometimes disappointing in terms of achieved data rates and reliability. From those separate experiences, it is difficult to point out which manufacturer/model is the most suitable for long term observatories. In other respects, recent progresses in digital signal processing can make hope that data rates of several thousands of bits per second are achievable for vertical links on a routine base. Those data rates are displayed by most manufacturers, but it is impossible to discriminate actual performances from commercial arguments without a series of in-depth tests.

The objective of the deployment during cruise P386 was to evaluate and select the most appropriate acoustic modems on the world market for fixed, long term observatory applications. This task was broken down into three steps:

- 2007: Paper selection of 5 modems on the world market, the primary selection criterion being the quantity of energy necessary to transmit one bit at a given distance.
- 2008: Short term transmission tests aboard R/V *L'Europe* (Commodac cruise), between a sea-bed station (MAP2) at 2200 mbsl (meters below sea level) and the ship. Among the five manufacturers selected at step 1, only three were fully ready for the integration deadline before the cruise.
- 2009-2010: Long term tests of the models ranked 1 and 2 at step 2, between the MAP2 seabed station at 2200 mwd and a surface buoy (Borel). The deployment of MAP2 and Borel was scheduled for the P386 cruise, and recovery is planned for early 2010 with R/V *L'Europe* (see below).

The testing principles are described in more detail in the Methods section of this report (Chapter 6.1).

Long-term data on sediment dynamics

Direct observation of turbidity currents were seldom obtained, and time series data are even less common. The Var system is probably the only system world-wide where regular observations (appx. every 2 years) have been made in order to observe, quantify and characterise turbidity events. The observations have been carried out by the means of moorings where different devices are installed. Data obtained with "classical" moorings concern turbidity (not calibrated), pressure, temperature, punctual current meters and sediment traps. This type of moorings have shown to be efficient to observe turbidity currents but not efficient enough to quantify and characterize them. At the same time, this moorings are intrusive and hence subject to the action of the turbidity currents. Entrainment

by currents may displace the moorings or at least affect their stability and the accuracy of the measurements obtained.

The main purpose of the ongoing study is to test and improve new tools for the direct observation and quantification of turbiditic events along the Var canyon. For this purpose, the "classical" IFREMER moorings where improved by the integration of an ADCP current meter able to record the vertical structure of the currents up to several tens of meters and hence its values near the canyon bed where the current magnitude is supposed to be the highest. Other systems include mounted 75 kHz ADCP current meters 300 m above the canyon's talweg to shed light on the vertical structure and hence the thickness of the turbiditic events.

The various instruments deployed along the pathway of the Var canyon is described in some detail in Chapter 6.2 (see below).

5. Logistical approach

(A. Kopf)

Given the number and size of instruments to be deployed during expedition *Poseidon* P386, we split the shiptime into two stretches.

Leg A was mostly dedicated to the long-term deployments in the Var Canyon area and the mooring and corresponding sealevel buoy further SE (see Ch. 6.1 and 6.2 below). Given that deck operations involving to move/lift/deploy heavy gear are restricted to daylight hours, we operated the CPT lance, Rn counter as well as the lightweight ROV during the remainder of the time. Given that the majority of the instruments got deployed during the first couple of days (e.g. Borel buoy, MAP2 station, Aniitra, Ibsen and Peer Gynt instruments including a number of anchor weights), Leg A spanned only from June 20-26, 2009 before returning to the mid-cruise port call. For most of the time, we operated R/V *Poseidon* very close to Nice shore (Fig. 13).



Fig. 13: R/V Poseidon in front of the Promenade des Anglais, Nice.

For Leg B, we then loaded two piezometer probes of >6 m length as well as a seafloor unit (Seamonice) for communication with one piezometer. The second leg went from June 26 – July 6, 2009. It was mostly dedicated to gravity coring, CPT and piezometer deployments, and Rn measurements. Since the deployment of the piezometer v1 plus Seamonice unit

required a scuba diver, we arranged for a second vessel aside of R/V *Poseidon* to optimise operations.

Three research scuba divers joined expedition P386 for the period June 27-30. They were based onshore and came out to the NAIL study area with the rented platform *Poseidon III* during the day (Fig. 14). During that period, *Poseidon III* also hosted the Rn counter and a scientist accompanying the measurements. Vice versa, the divers were picked up by the dinghy of R/V *Poseidon* regularly for scientific discussion and handing over rhizon samples taken from the shallowmost sub-seafloor.



Fig. 14: Diving vessel Poseidon III operating in front of NiceAirport.

6. Methods

6.1. Long term tests of acoustic modems for subsea observatories

(J. Blandin, J.-P. Brulport, P. Crassous, G. Gruyader, J. Legrand, P. Pichavant)

The two manufacturers selected for these tests are Evologics GmbH (Berlin) and Sercel UAD (Brest). The modems under test are mounted between the top of a seabed station (MAP2), deployed at a water depth of 2200 m, and the keel of a surface buoy (Borel) moored within acoustic range of MAP2 (see schematic layout in Fig. 15). On MAP2, an electronic unit (Costof) sequences periodic emissions of data files of various sizes, alternatively through each modem under test. The energy quantity consumed on MAP2 for each transmission is measured and transmitted to the buoy. The buoy sends the result of the transmission (number of errors if any, consumed energy) to shore via the Iridium satellite system. The functioning parameters of both the surface and seabottom modems can be modified at any moment from shore if necessary. The buoy periodically performs local measurements (atmospheric pressure, wind speed, X, Y inclination) that are logged on shore and can be correlated with the acoustic data transmission results.

The scheduled duration of these tests is seven months.

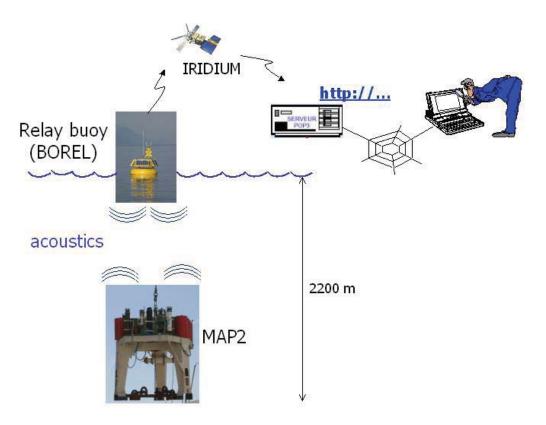


Fig. 15: Sketch showing the long term testing principle using the Borel buoy.

6.2. Sediment traps, ADCP and current meters

(R. Jacinto Silva, J. Legrand, P. Pichavant, J. Blandin, G. Gruyader)

The main purpose of the on-going study is to test and improve new tools for the direct observation and quantification of turbiditic events along the Var canyon. For this purpose, the "classical" moorings (termed "Peer Gynt"; see Fig. 16) were improved by the integration of an ADCP current meter able to record the vertical structure of the currents and hence its values near the canyon bed where the current magnitude is supposed to be the highest. This reference current meter is placed 25 m above the talweg and should provide directional information of the currents.



Fig. 16: Photograph of part of the Peer Gynt mooring prior to deployment.

Complementary to Peer Gynt, a new non-intrusive mooring system has been deployed for test purposes. The "Aniitra" system (Anchorage of Non-Intrusive Instruments to Track and Record under-water Avalanches; Fig. 17) is a buoyant structure mounted with a 75 kHz ADCP current meter may stay at a vertical position (300 m above the canyon's thalweg) higher than the expected current events. The Aniitra system should provide the vertical structure and hence the current thickness of the turbiditic events, a crucial information to

quantify the events and constrain any numerical modelling of these events. It hence improves the resolution of the vertical structure of the current, but it is unable to give directional information. It is henced to be deployed in conjunction with Peer Gynt.

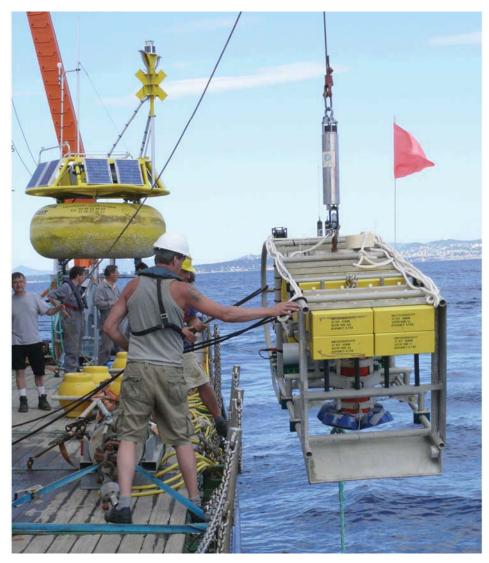


Fig. 17: Photograph of the Aniitra system during deployment.

In order to obtain a Lagrangian observation of the currents together with a longitudinal capability of the currents to entrain and transport, an Inflow Buoy for Sediment Entrainment (Ibsen; Fig. 18) has been developed. Its fairly negative buoyancy becomes positive in presence of a denser fluid, as it is supposed to be the case in sediment-laden flows. The Ibsen buoy records its vertical position by the means of a pressure gauge. Assuming its displacement follows the canyon axis, any vertical position (water depth) is supposed to be associated with a single position along the canyon. The quantification of its displacement through time provides an estimation of the Ibsen velocity.



Fig. 18: Photograph of the Ibsen mooring prior to deployment.

Sediment traps (Fig. 19) are installed in the "classical" moorings at VV and VA. A rotational reservoir replaces one of the 24 bottles every 7 days. This means that sediments traps are a very low frequency devices providing only a qualitative information on particles vertical fluxes. By themselves they are not very accurate, but they provide only information available about particles in the suspension. When correlated to current velocity and Var river runoff, particle fluxes are objectively the effective signature of the turbiditic nature of the measured currents.



Fig. 19: Photograph of a sediment trap (upper centre) used during expedition P386.

6.3. Shallow water methodology

6.3.1. Echosounder

(A. Kopf, S. Stegmann)

During cruise P386, all research activities could be based on earlier quality bathymetric charts recorded with multibeam systems from KONGSBERG MARITIME (formerly SIMRAD) during R/V *Meteor* cruise M73/1 (Kopf et al., 2008) as well as data acquired by our colleagues from IFREMER beforehand. The only area not mapped prior to cruise P386 was the portion immediately south of Nice airport, because larger vessels have too much draught to navigate safely that close to shore.

With R/V *Poseidon*, we were able to utilise the Pilot echosounder by Krupp-Atlas-Elektronik, which is designed for extremely shallow water depths (< 50 m), ideally even for 20 m and below. The x, y data from this system were logged every second. As a consequence of the time-consuming station work, in particular during deployments of the piezometers, longer CPT tests, Rn counting, and ROV dives, we tried to gather a comprehensive data set north of the existing bathymetric chart.

6.3.2. Underwater video surveys (ROV, scuba dives)

(A. Kopf, T. Pichler, R. Price, M. Seydel)

Remotely Operated Vehicle SPY

The remotely-operated vehicle (ROV) "Spy" is a shallow-water device operable down to 250 m water depth (Fig. 20). Its depth range is largely limited by the total length of the cable, which is supplied on a separate winch; for the Nice Airport area, however, this does not pose a problem given attr depths < 100 m. The ROV is powered by either 230 DC or 350 DC and is controlled by a console unit with VHS TV screen and a keyboard for the operations. The ROV is equipped with two underwater cameras. One is looking towards the ground while the other one is looking forward. For operation at night or in areas with poor visibility, it is equipped with a pair of headlights. Two turbines at the back control forward/backward movement, while another pair of subvertically mounted turbines control submersion or ascent (Fig. 20). Its total weight of 60 kg (in air) has to be balanced depending on water density and payload. During cruise P386, it was largely used for

surveying the seafloor to find morphological anomalies or evidence for seepage (using the cameras).

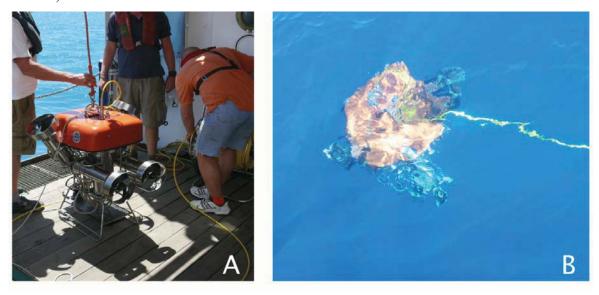


Fig. 20: ROV "Spy" for surveying the sea bottom for locations of fluid seepage; (a) on deck, (b) in the water.

For part of P386 Leg B, research scuba divers joined the P386 science party to do video surveying, deploy *in situ* temperature probes (see Ch. 6.3.3. below) and take fluid samples out of the shallow sub-seafloor.

6.3.3. In situ temperature measurements

(A. Kopf, T. Pichler)

On cruise P386 the *in situ* temperature gradients were measured with miniaturised autonomous temperature data loggers (MTL, Fig. 21). For technical specifications and detailed information, refer to Pfender & Villinger (2002).

Parameters of autonomous temperature data loggers:

Instruments serial no.: sediment and water temp. logger: 18543-65C, -67C, -68C, -

70C, -75C, -77C, -78C, -79C

Sample rate: 1 min

Spacing: 1 m, 2 m, 3m, etc. below the weight set;

One water temperature sensor at weight set with sensor tip

looking up.

Measurements (seafloor penetration) were planned to be carried in two different ways:

(i) Stick MTL into shallow subseafloor (10-30 cm), performed by the scuba divers. The probes are marked with flags along a line and remain fully sediment-covered in the sub-

seafloor for a period of several days before they get recovered. For P386, the planned MTL arrays were within the hydologically most active portions of the Nice slope.

(ii) Mount MTL to gravity core barrel and get gradient at 1m – spacing as well as reference temperature in the water column. The probe remains in the seafloor sediment for several minutes to allow for some dissipation of artificial frictional heat from inserting the gravity corer.





Fig. 21: Miniature temperature logger (MTL) and its housing with supporting fin along the steel barrel of e.g. the gravity corer (top) and deployed in the shallow sediment by a scuba diver (bottom).

6.3.4. In situ CPT testing

(S. Stegmann, A. Kopf, A. Förster)

On R/V *Poseidon* cruise P386, we used the lightweight MARUM free-fall CPTu probe (see Fig. 22a, and Stegmann et al., 2006). Cone Penetration Testing (CPT) is an effective method for *in situ* measurements of these geotechnical parameters with one instrument (Lunne et al., 1997), namely sedimentary strength (tip resistance, sleeve friction), pore pressure, tilt and acceleration. For these measurements, the CPT system relies on 15 cm² standard industry piezo-cone (Fig. 22b) with the sensors at the tip and a pressure housing containing a microprocessor at the top. In addition, deceleration and tilt are monitored for vertical profiling of the penetrated sediment column. The lightweight (40-170 kg), shallow water (100-200 m depth) lance works completely autonomously with a volatile memory and battery package. It has exchangeable CPT probes at its tip to accommodate for the various geological settings it is used in.

Instrument

The lightweight free-fall CPT (FF-CPT) instrument for shallow marine use consists of an industrial 15 cm² piezocone and a water-proof housing containing a microprocessor, volatile memory, battery, and accelerometer (Fig. 22; see Stegmann et al., 2006 for details). Strain gauges inside the probe measure the cone resistance and sleeve friction by subtraction. A single pore pressure port (u₂) is equipped with an absolute 10 MPa (CPT probe 1) and 20 MPa (CPT probe 2) pressure sensor. An inclinometer is used installed to monitor the penetration angle at +/-20° relative to vertical. Four different accelerometers with different ranges (+/- 1.7g, +/- 5g, +/- 18g, +/- 100g) provide information about the descent velocities and deceleration behaviour of the instrument upon penetration. These data allow the researcher to calculate velocity penetration depth during multiple deployments by $1^{\rm st}$ and $2^{\rm nd}$ integration. The aluminium pressure housing tolerates 2 MPa confining pressure (ca. 200 m water depth) and hosts the power supply and microprocessor. Frequency of data acquisition is variable and depends on the data logger used (see next paragraph). Binary data are temporarily stored on a Micro Flash Card and then downloaded to a PC. The two non-volatile battery packs available provide performance times of about six and twelve hours, respectively. The length of the lance may be varied from 0.5 m to a max. 6.5 m depending on what type of sediment is anticipated. The extension is accomplished by adding 1m-long metal rods and internal extension data/power cables within them. The weight of the instrument thus ranges from ca. 45 kg to max. 110 kg. If deep penetration is desired, modular weight pieces (15 kg each) can be mounted to the pressure housing at the top of the instrument, then reaching a max. 170 kg. The instrument is deployed pogo-style and remains in the seafloor for about 20 minutes for individual measurements.

Modes of deployment

The FF-CPT instrument was used with two different types of CPT-probes, two different data logging units, and in two types of deployments (profiling vs. pore pressure dissipation). During cruise P386, the probe was generally deployed in 3.5 m long mode with 4 weight sets (i.e. 60 kg) attached.

As for the CPT probe, two systems supplied by GEOMIL (Alphen, NL) were used. The first, more sensitive unit is only designed for 100 m water depth (i.e. 1 MPa absolute pore pressure transducer, 25 MPa cone resistance strain gauge, 0.25 MPa sleeve friction strain gauge). The less sensitive 200 m tip comprises a 2 MPa absolute pore pressure transducer and 100 MPa (cone resistance) and 1 MPa (sleeve friction) strain gauges.

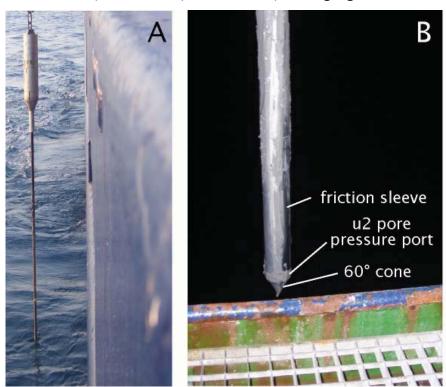


Fig. 22: Shallow-water FF-CPTu instrument (left) and detail of the CPT probe (right).

Two different types of microprocessor data recording units were used. The older model contains a TigerBasics microcontroller which logs the data at 40 Hz. It was usually utilised for tests aiming at the shape of the pore pressure dissipation curves (>> 30 mins. deployment

time). In contrast, a recently developed 1000 Hz AVISARO microcontroller was utilised during deployments, which largely aimed at the vertical profiling of the sedimentary succession. This usually takes less than 1-2 secs. and at the high sampling rate, provides the user with data of a vertical resolution < 1 cm thickness. The controllers are directly linked to the two fundamentally different modes of deployment. The first (type A) aims at a high-resolution vertical record of crucial *in situ* sediment physical properties. The probe is veered at 1.2 – 1.5 m/s winch speed and then dynamically decelerated until its terminal depth of several meters sub-seafloor. The instrument is recovered immediately after the probe came to a complete halt. The second approach (type B) is initially similar, however, aims at the recording of the pore pressure evolution once the instrument is stuck in the sediment. Pore pressure dissipation is usually recorded for 30 mins. (assuming the ship can be held at the location for that long), occasionally even for 60 mins. Given that R/V *Poseidon* does not have a dynamic positioning system, the majority of the deployments were either type A or B, but avoided dissipation periods exceeding 30 mins.

6.3.5. Piezometer deployments

(P. Pelleau, R. Approuial, S. Stegmann, A. Kopf)

Owing to the interesting hydro-geological processes in the study area, the *in situ* monitoring of pore pressure and temperature are a major focus of the P386 research. In addition to the deployment of MTLs (Ch. 6.3.3.) and CPT profiling (Ch. 6.3.4.), those key physical properties were measured in different depth levels in the sub-seafloor sediment using piezometer probes developed by IFREMER. The IFREMER piezometer is a free-fall device composed of a lance and an upper body consisting of the power supply, data storage unit and a package of recoverable weights.

The IFREMER piezometer is a modular free-fall device composed of a lance and an upper recoverable part with pieces of weights, power supply and data acquisition (Sultan et al. 2007). It is already existing in a second generation instrument Piezo v2, which has been improved in a number of places compared to the earlier v1 version. The overall design is schematically shown in Figure 23.

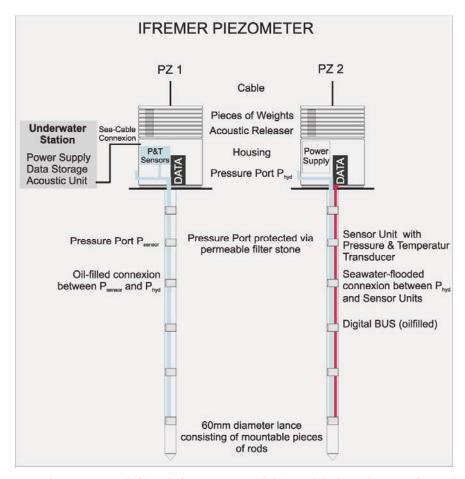


Fig. 23: Piezometer v1 (left) and piezometer v2 (right) used during R/V *Poseidon* cruise P386.

Depending on the requirements for the installation of the piezometer in a given location, the instrument can be set up in different configurations. The 60 mm-diameter lance consists of single rods (0.75m or 1.50m) intersected by sensor packages. The spacing and length of rods between the "piezomodules" define the depth levels where pore P and T are measured. The maximum total length of the lance is limited to 10 m for stability reasons. Each module contains a sensor package comprising a KELLER pore pressure transducer and a standard temperature sensor. Pore pressure is measured differentially with a resolution of \pm 0.2 kPa by coupling the pore pressure at the of each sediment level with the open seawater (hydrostatic reference) by a tubing. The sampling rate can be set up individually before each measurement, with the maximum rate being 1 Hz.

In the first generation (Piezo v1) the pressure transducer of each port is installed at the top of the instrument where the hydrostatic reference port is situated, while the lower port is connected via tubing along the lance (Fig. 23, left). The disadvantage is the compliance of the tubing in which gas may be trapped during deployment. The advancement in the second generation instrument (Piezo v2) is the direct contact of the pressure

transducer with both the formation and the hydrostatic reference because the transducer has one port as close to the formation as physically possible, whereas the second port points towards the inside of the probe through which open flow of seawater over the complete length is achieved (Fig. 23, right). Saturation with seawater of both systems is accomplished prior to each deployment by dipping the instrument for 20 minutes over its complete length into the water at moderate depth above the seafloor. For the final stage of the deployment, the instrument is lowered with the winch to the seafloor and the lance penetrates the sediment using its own mass.

The piezometer can be used for short-term (hours to days) as well as long-term (weeks to months) installations. For short-term measurements, the instrument is usually deployed on different locations without recovery on deck of the vessel in "yoyo-style" (e.g. Sultan et al. 2008). For long-term monitoring, the piezometer can be connected to an underwater-station providing continuous power and additional data storage capacity. An acoustic communication module of the underwater-station allows the transfer of the data without recovering the piezometer. During cruise P386, we had one piezometer v1 funded by IFREMER) and one piezometer v2 (funded by MARUM) available for long-term deployment. Regarding the setup of the two instruments, mode of deployment (yo-yo vs. long-term), sampling rates and locations, see Ch. 7.3.5 below.

6.3.6. Gravity coring and sediment description

(A. Förster, T. Fleischmann, K., Weber, S. Stegmann, A. Kopf)

In order to recover sediment cores, two sampling systems were used during P386: (i) a gravity corer with tube lengths of 3 to 6 m and a weight of approximately 2 tons (Fig. 24a), and (ii) a light-weight "bobcorer" of only 150 cm length and smaller diameter for use in very shallow water (Fig. 24b). Before using the coring tools, the plastic liners (placed inside the steel tubes in case of the gravity corer, but representing the outer cylinder in case of the bobcorer) have been marked lengthwise with a straight line in order to retain the orientation of the core for potential paleomagnetic analyses.



Fig. 24: Gravity corer (a) and Bobcorer (b) on board R/V Poseidon.

Once on board, the sediment cores were cut into sections of 1 m length, closed with caps on both ends and labelled according to a standard scheme (Fig. 25). By definition, the half core with the marked line was stored as archive half (after having passed the Multi-Sensor Core Logger – see Ch. 6.3.6.3.), while description, sampling, etc. were carried out on the remaining half. For the detailed procedures each working half core underwent, see below.

Inscription:

GeoB 7101-1 212 112 GeoB 7101-1 112 212 Work liner 1-1017 211 1-1017 212 7101-1 212 W caps GeoB 7101-1 7101-1 Archive GeoB 7101-1 Work cutting

Fig. 25: Scheme of the inscription of gravity core segments used during P386.

site number: GeoB 7101-1 core depth: 112-212 cm

orientation for paleomagnetic sampling

Sediment description

Split gravity cores were photographed and described from a largely sedimentological standpoint. Grain size and composition of sediments were determined mainly visually using a simple hand-lens, HCl-testing and analyzing smear slides of dominant lithologies under a cross-polarizing microscope in accordance with Rothwell (1989). The size of grains was assessed based on Wenthworth's (1922) classification. The colour of the material was determined visually on board using Munsell's colour chart nomenclature, and also has been studied spectrophotometrically after the cruise on the Multi-Sensor Core Logger (MSCL; see Appendix 10.3). For each core, a composite one-page core log sheet was compiled. It shows core photographs next to a graphical core log and gives information on redeposition-/event layers (i.e., sand layers, volcanic ash layers or clear evidence for mass movement deposits, such as mud clasts in muddy or sandy matrix, tilted beds and repetition of strata), bioturbation and the assigned lithological units in three different columns. The core log is combined with results from the fall cone penetration test (see below). A wide variety of features, such as sediment lithology, primary sedimentary structures, bioturbation, softsediment deformation, and coring disturbance is indicated by patterns and symbols in the graphic logs. A key to the full set of patterns and symbols used on the barrel sheets is shown in Figure 26. The symbols are schematic, but they are placed as close as possible to their proper stratigraphic position. All core descriptions are provided in Appendix 10.2 (see below).

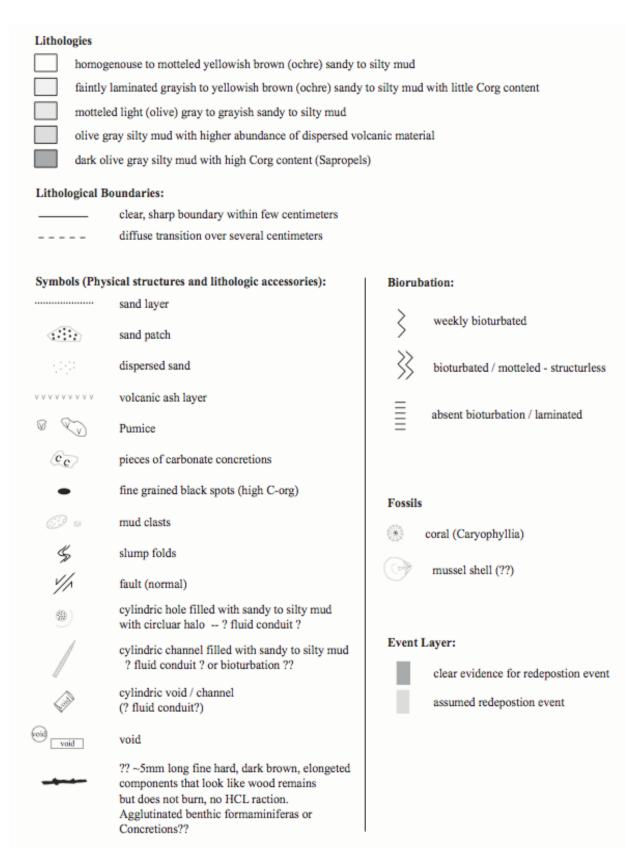


Fig. 26: Key of symbols for barrel sheets of gravity core description.

6.3.7. Physical properties

(A. Förster, T. Fleischmann, S. Stegmann, K. Weber)

During cruise P386, shipboard physical properties measurements were restricted to falling cone penetration tests and vane shear tests on the working half of the core. Since no container with a Multi-Sensor Core Logger (MSCL) could be placed on board RV *Poseidon*, these measurements on the undisturbed archive half of the cores were carried out immediately after the cruise at MARUM Bremen. A description of the instrument is given below (Ch. 6.3.7.3.).

6.3.7.1. Cone penetrometer

The geotechnical properties along the sediment cores were determined according to British Standards Institutions (BS1377, 1975). A Wykeham-Farrance cone penetrometer WF 21600 (Fig. 27a) was used for a first-order estimate of the sediment's stiffness. For the measurement, the metal cone was brought to a point exactly on the split core face. A manual displacement transducer was then used to measure the distance prior to and after release of the cone (i.e. penetration after free fall of the cone). Precision is 0.1 mm of displacement. The distances measured can then be translated into sediment strength (see Hansbo, 1957).





Fig. 27. (a) Falling cone penetrometer and (b) PC-interfaced IFREMER double-vane shear device used on the split core surface.

A falling cone penetrometer with a defined weight (80.51 g) and geometry (30 $^{\circ}$ cone) was used by Hansbo (1957) during a detailed study of the relationship between the cone penetration and soil strength. The undrained shear strength s_u can be calculated from the

variables mass and tip angle of the falling cone, gravity g, penetration depth d and the cone factor k via the "cone factor". Wood (1985) calculated from fall-cone and miniature vane tests average values of cone factors (in our case k=0.85 for a 30° cone). The undrained shear strength can then be calculated using the equation $s_u = (k*m*g)/d^2$.

Shore-based laboratory testing will include ring shear experiments as well as dynamic triaxial shear tests to obtain residual strength and rate-dependent frictional properties as well as the liquefaction potential of the materials recovered.

6.3.7.2. Vane shear testing

In addition to the Cone Penetrometer a double vane shear apparatus by GSC ATLANTIC was used for more information about sediment stiffness and residual shear strength (Fig. 27b). The distance between the two vanes is 15 cm. For the measurements, four-bladed vanes (L = 12.5 mm, h = 6.25 mm, d = 12.5 mm) were inserted into the split undisturbed core faces and rotated at a constant rate of 90°/min. Data are logged via an interface module (GSC ATLANTIC) using the Testpoint software package.

A spring transmits the rotation at the vane. The torque required shearing the sediment along the vertical and horizontal edges of the vane. The undrained shear strength, s_U depends on the torque T, the vane constant K, the maximum torque angle at failure σ and the spring constant B that relates the deflection angle to the torque (Blum, 1997). The vane constant, K is a function of the vane size and geometry and was used during the measurements with $K=\pi^*d^2*(h/2)+\pi^*(d^2/6)$ for full dipping vanes. The undrained shear strength can then be calculated using the equation $s_U=T/K$. Shore-based laboratory testing will include ring shear tests to obtain residual strength and rate-dependent frictional properties of the materials recovered.

6.3.7.3. Multi-sensor core logger

The *GEOTEK* MSCL device at MARUM Bremen combines three sensors on an automated track (see schematic diagram in Fig. 28). The P-wave velocity, gamma ray attenuation (bulk density), and the magnetic susceptibility were recorded, and from this data the fractional porosity and impedance were calculated. RGB images were also produced with a full color digital line scan imaging system. Magnetic susceptibility, bulk density, and line scan photography were generally measured on all cores.

Magnetic Susceptibility

Magnetic susceptibility was measured with a Bartington point sensor MS2 using an 80-mm internal diameter sensor loop (88-mm coil diameter) operating at a frequency of 565 Hz and an alternating field of 80 A/m (0.1 mT). The sensitivity range was set to the low sensitivity setting (1.0 Hz). The sample period and interval were set to 2 s and 4 cm, respectively, unless noted otherwise. The mean raw value of the measurements was calculated and stored automatically. The quality of these results degrades in XCB and RCB cores, where the core may be undersized

and/or disturbed. Nevertheless, general downhole trends are useful for stratigraphic correlations. The MS2 meter measures relative susceptibilities, which have not been corrected for the differences between core and coil diameters.

Gamma-Ray Attenuation

Bulk density was estimated for split core sections as they passed through the GRA bulk densiometer using sampling periods and intervals of 2 s and 4 cm, respectively, unless noted otherwise. A thin gamma beam from a Caesium-137 source with energies around 0.662 MeV is passed through the core and the relative intensity of this beam can be used to measure the gamma density. These photons are scattered by electrons in the core and loose some of their energy. To determine the gamma density the number of unscattered electrons is measured by counting photons with the same principle energy as the photon source. The gamma density of an aluminum billet of stepped thickness is used to obtain calibration equations to convert gamma density into actual density values.

<u>P-Wave Velocity</u>

The P-wave velocity is measured at 4-cm intervals and 2-s periods using two PWL transducers. The PWL measured P-wave velocity across the unsplit core sections. In order to determine the P-wave velocity, the PWL transmits 500-kHz P-wave pulses through the core at a frequency of 1 kHz. The transmitting and receiving transducers are aligned perpendicular to the core axis while a pair of displacement transducers monitors the separation between the P-wave transducers. Variations in the outer diameter of the liner do not degrade the accuracy of the velocities, but the unconsolidated sediment or rock core must completely fill the liner for the PWL to provide accurate results. During this measurement good acoustic coupling between the core liner and transducer is achieved by adding water to the contact points.

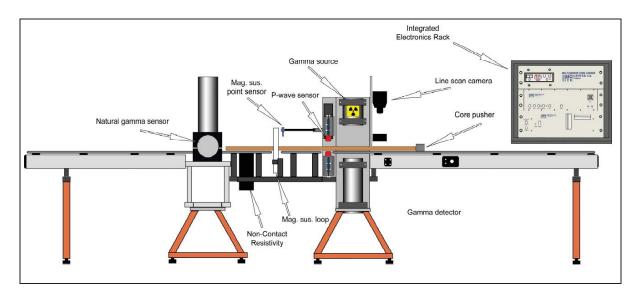


Fig. 28: Schematic of the Geotek Multi Sensor Core Logger (MSCL).

6.3.8. Pore water chemistry

(T. Pichler, S. Pape, S. Hammerschmidt, R. Price, M. Seydel)

Pore water sampling

All gravity cores were taken by plastic liners and cut into 1 m segments on deck. To prevent a warming of the sediments on board, the sediment cores were immediately transferred into the cooling room after recovery and maintained at a temperature of about 4°C. The wet sediment was exposed by means of cutting a small 'window' in the plastic liner at an interval of 25 cm. Eh and pH were measured directly in the sediment using punch-in electrodes before the pore water was extracted (Fig. 29a). From most of the cut segments of the cores 5 ml syringe samples of wet sediment were taken for methane analysis. The pore water was then extracted by means of rhizons (pore size 0.1 µm) (see Fig. 29b). The gravity cores were each processed in this way within a few hours after recovery. Depending on the porosity of the sediments, the amount of pore water recovered ranged between 4 and 20 ml. Solid phase samples of the majority of cores were taken for total digestions, sequential extractions and mineralogical analyses at 25 cm intervals, kept in gastight glass- and heavy plastic bottles under an argon atmosphere and stored at 4°C.

Pore water analyses of the following parameters were carried out during this cruise: Eh, pH, ammonium, alkalinity, iron (Fe²⁺). Salinity was also measured using a conductivitymeter for selected cores where fluid freshening was suspected. Eh and pH, as mentioned earlier, were determined with punch-in electrodes before pore water was extracted. Ammonium was measured using a conductivity method. Alkalinity was calculated

from a volumetric analysis by titration of either 0.5 or 1 ml of the pore water samples with 0.01 M HCl, respectively. For the analyses of dissolved iron (Fe²⁺), subsamples of 1 ml were taken from pore water extracted by rhizons, immediately complexed with 50 μ l of "Ferrospectral" and determined photometrically. Salinity was measured using a conductivity probe placed directly into the pore water samples.

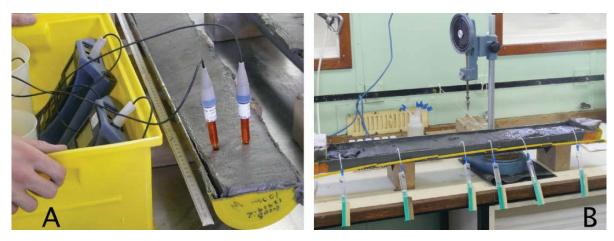


Fig. 29: (a) pH measurement and (b) rhizon pore water extraction in split working half of the gravity core.

For further analyses at the University of Bremen aliquots of the remaining pore water samples were diluted 1:10 and acidified with ultra pure HNO₃ for determination of cations (Ca, Mg, Sr, K, Ba, S, Mn, Si, B, Li) by ICP-OES. Additionally, 0.6 mL of a ZnAc solution was added to a 1.5 mL subsamples of the pore to fix hydrogen sulfide as ZnS for later analysis. Finally, all remaining pore water was stored at 4 °C for additional analyses at the University of Bremen. A complete overview of sampling procedures and analytical techniques used on board and in the laboratories at the University of Bremen is available on http://www.geochemie.uni-bremen.de/.

To test for direct venting of submarine groundwater into the shallow water portion of the study site pore waters were collected every 10 m along the transect prepared with MTLs (see Ch. 6.3.3. above). These were collected into 60 mL syringes by inserting the syringes approximately 5 cm into the sentiment and slowly pulling the plunger. The entrainment of fine particles could not be avoided, nevertheless, these were filtered out once the syringes were passed to thegeochemists onboard R/V *Poseidon* for further analyses. After filtration, these samples passed the same analytical routines as those extracted from the gravity cores.

6.3.9. In situ Radon measurements

(A. Mayer, P. Henry)

Overview

Radon-222 is a radioactive noble gas produced by decay of radium-226. The half-life of radon is 3.8 days, which implies that this gas is never "far" from its source (radium). Several studies have shown that groundwater often presents high radon activity. This is due to the presence of radium at the surface of the aquifer solids. Groundwater may reach in some cases an activity of millions of Bq/m3. By contrast, due to atmosphere degassing and decay (or more correctly the distance from the radon source), radon activity in seawater is very low (10-14 Bq/m3). Seawater affected by seepage of groundwater might thus be detected because of the large contrast in radon concentrations. In addition, steady state radon inventory in water column implies that radon natural decay is sustained by radon production, thus radon concentrations might be readily translated into water fluxes.

Radon concentrations measured at different depths along a vertical profile off the Nice airport might be used to detect local seepage of groundwater, and estimate involved seepage flux once knowing radon activities in a) groundwater and b) offshore seawater.

Methods

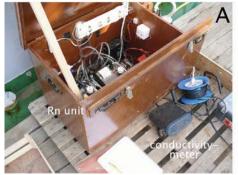
The available (up-to date) radon measurement methods developed by Adriano Mayer (CEREGE) allows precise measurement of radon activity *in situ*, without the need for collection a sample. Two alpha spectrometers, gas-exchange cell, water stripping system might be installed in a water resistant wooden box (1.2 x 0.6 x 0.8 m, about 40 kg; Fig. 30a), allowing continuous measurement of radon, with 30 minutes count integration for each acquisition. Once launched, the apparatus does not require presence of personnel, and may be used on a small (3-4m long) rubber craft (Fig. 30b). Typically, the measurement in one spot - one depth requires: 5 minutes for installation, 30 minutes for purge of previous measurement (might be done earlier), 1h 30 minutes acquisition (for six values, having 2 spectrometers). More precise values are obtained with longer acquisition time.

During acquisition, a 12V immersion pump (turbine) with attached flexible pipe produces a continuous water flow of about 1.5-3 litre/minute from the desired depth to the gas exchanger in the wooden box. The seawater is discharged to the sea after radon degassing (no sample is retained). In addition to the Rn counts, conductivity measurements are carried out in the water cylinder surrounding one of the filters oof the pump. This way the

researcher can tell immediately whether fresh, brackish or saline water is entering the system, and can hence relate this information to the Rn counts. The latter are estimated at real time by an acoustic signal. The precise protocol of the Rn counts is measured by two The sampling depth is attained maintaining the flexible pipe and 12V cable in vertical position, tied to a rope with a weight attached to its lower end. During cruise P386, both concrete weights as well as a metal weight (both appx. 10-15 kg) were used to keep the hose close to the seafloor. A buoyant device was tied to the nozzle of the hose to ensure that the open end of the hose is not dipped into the sediment-water interface (Fig. 30c).

Application

Rn measurements were usually not carried out as a stand-alone measurement, but complemented other deployments at the same station (CPT, gravity core). Depending on the time spent for the measurements, radon concentrations could be measured on station from the deck or dinghy (see previous paragraph and Fig. 30), or continuously near the sea surface while the dinghy or the ship is slowly sailing (max. 3-5 knots). In the latter case, a horizonztal transect of radon concentrations of the near surface seawater is obtained, which would nicely supplement the vertical Rn profiles (see above). If the radon background concentration in the local seawater, far from the radon source, as well as the concentration of radon in the groundwater are known, under given assumptions a groundwater seepage rate may be calculated. The strategy for the Nice slope was (a) to collect sufficient data near the sealevel as well as in the deeper water to determine a "background Rn concentration" for the ocean water in the Nice landslide vicinity, (b) to carry out measurements in the estuary of the river Var to determine the "input Rn concentration", and (c) to measure Rn concentrations very close to the seafloor in locations where (gas) flares were seen in 3.5 kHz seismic profiles (based on Sultan et al., 2008) or in locations where previous workers found evidence for freshening of the seawater (Guglielmi & Prieur, 1997; Kopf et al., 2008, 2009).





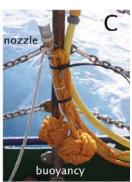


Fig. 30: (a) Setup of the Rn counters, pumps, filters and conductivity meter in the wooden box on the deck of R/V *Poseidon*; (b) Rn setup on the dinghy; (c) buoyancy device attached to lower end of hose. See text.

Figure 30 shows the two types of deployment strategies used on P386. The first approach has the Rn measuring unit on deck of R/V *Poseidon*, close to the railing. The hose with the nozzle is lowered either to a few meters sub-sealevel depth (to determine the background water body Rn concentration) or lowered to the seafloor where a buoyancy device hinders sucking of sediment suspension into the pump, filters and Rn counter (Fig. 30a). Although this is the least laborious setup, the disadvantage is the fact R/V *Poseidon* usually does not remain on station for very long periods of time (only 20 mins. for gravity coring, and appx. 40 mins. for a CPTu test). Hence, the Rn measurements from deck served mostly the collection of background data (see Type "a" above). Only in spots where mid-term pore pressure dissipation tests were planned with the CPTu instrument (see Ch. 6.3.4. above), the hose was lowered all the way to the seafloor to get signals at fresh water sites. For longer term Rn counts, the unit was either placed onto the dinghy (Fig. 30b) or onto the diving platform *Poseidon III* (see Fig. 14). Both small watercrafts aimed for fresh water locations only, so that measurements of up to >2 hrs. got achieved.

7. Preliminary Results

The preliminary results from cruise P386 can be divided into three major topics:

(i) Technological developments regarding acoustic data communication for seafloor observatories, (ii) deployments for sedimentological research in the Var Canyon turbidititic system, and (iii) the wealth of geological, geotechnical and geochemical measurements and long-term deployments in the Nice Airport slide area. Figure 31 provides a map where the three objectives were carried out. In the following, results from each of them are documented in consecutive chapters.

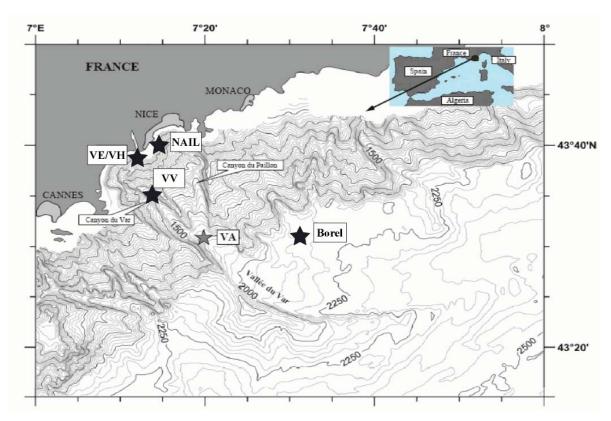


Fig. 31: The study areas (black stars) visited during cruise P386. The main focus was on the Nice Airport Slide (NAIL), mooring deployments in the Var Canyon talweg (VE, VH, VV) and on long-term acoustic communication tests using the Borel buoy (Borel). VA is a point of a former deployment unrelated to P386.

7.1. Long term tests of acoustic modems for subsea observatories

(J. Blandin, J.-P. Brulport, P. Crassous, G. Gruyader, J. Legrand, P. Pichavant)

The MAP2 station (Fig. 32) containing the acoustic communication hardware was deployed on June 22, 2009. It was attached to a deep-sea cable with an acoustic release, then lowered from the ship to 2080 m below sea level, and was finally released and traveled the last 100 m to the seabed in free fall.



Fig. 32: The station MAP2 has been moved overboard, ready to be lowered.

The Borel buoy was deployed on June 22, 2009 by first bringing it out overboard at a point 4000 m below wind of the target point for the deployment. Afterwards, the 2400 m of line were paid out as the ship proceeded slowly towards the target point (see Fig. 33B). When all the line was paid out, R/V *Poseidon* continued its route until 650 m beyond the target point, where the 1900 kg of ballast were released overboard.

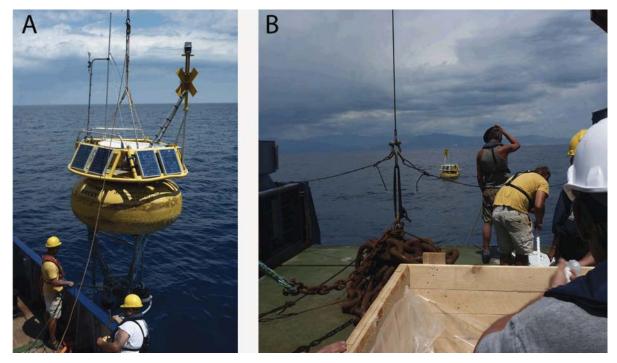


Fig. 33: The Borel buoy over the port side of R/V *Poseidon* (A) and when being released during paying out the mooring line (B). Note also ballast chain waiting to be deployed.

The link between MAP2, the Borel buoy and shore was established immediately after deployment of the buoy. During leg A of expedition P386, the modems from one manufacturer performed successful data transmissions every two hours, as scheduled. The top modem of the other manufacturer showed difficulties in receiving the messages transmitted from the bottom, even with a low data rate and high transmission power. The situation was described to the manufacturer who proposed to send a replacement electronic board to be installed in the waterproof container of the buoy transducer. This operation required good weather and the ability to lift the buoy back to the side on deck-level in order to retrieve the transducer from the buoy. This was achieved during leg B where a replacement board brought by the second science party was mounted successfully. However, tests during the second half of leg B failed to attest successful data communication between the second MAP2 modem, the buoy and the shore.

The Borel buoy and MAP2 station are scheduled to be recovered in January 2010 from R/V *L'Europe*, together with the Var Canyon observation devices (see next chapter).

7.2. Sediment traps, ADCP and current meters in the Var Canyon

(R. Jacinto Silva, J. Legrand, P. Pichavant, J. Blandin, G. Gruyader)

Current meters are placed at three locations along the Var canyon: VE, VH and VV (see Fig. 31). Moorings VH and VE were deployed to provide detailed information for the upper Var Canyon (see detailed map in Fig. 34). At location VH (Fig. 35a), three punctual current meters are installed at 15m, 25m and 35m above the canyon's talweg in order to provide the current structure at the head of the canyon. The mooring placed at VE (Fig. 35b) has a 300kHz ADCP 30 m above the talweg in order to provide a continuity to VH monitoring.

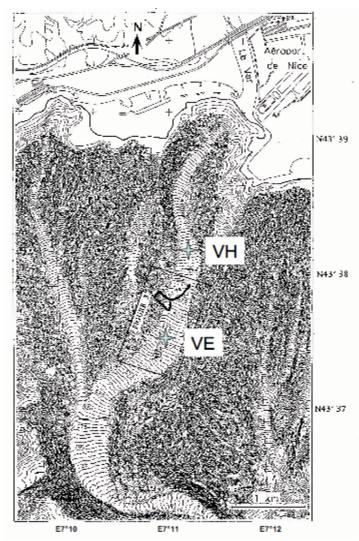


Fig. 34: Detailed bathymetric chart of the upper submarineVar Canyon with the locations of moorings VH and VE. See text.

The station VV is the place for a "classical" mooring with a control current meter placed 25 m above the talweg (Fig 35c). This current meters should provide a directional information of the currents. At the same location, the Aniitra mooring is deployed with its 75kHz ADCP current meter.

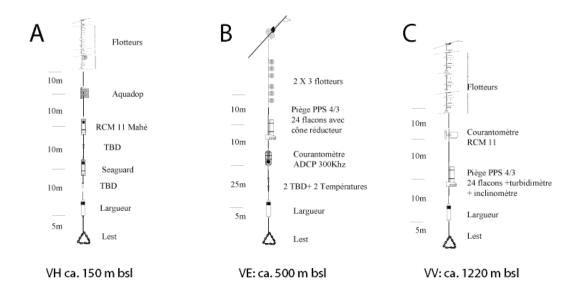


Fig. 35: Schematic diagrams of the moorings installed during cruise P386. Mooring VV contains the Aniitra unit (see Fig. 17). See Figs. 31 and 34 for locations of deployments.

7.3. Nice Airport landslide

7.3.1. Echosounder

(S. Stegmann, A. Kopf)

No multibeam swathmapping system was mounted on R/V *Poseidon* during cruise P386, and no gephysicists were part of the scientific crew. As a result, we were only able to record the echosounder depth and position at intervals of 1 minute using the shipboard DATAVIS system. These XYZ-coordinates are filtered regionally and serve to slightly extend the existing bathymetric charts by IFREMER and MARUM towards the shore. This was possible because R/V *Poseidon* has lower draft (4.5 m) than R/V *Meteor* (Kopf et al., 2008) or R/V *L'Atalante* (Sultan et al., 2008) when surveying the area two yers earlier. Postcruise, the P386 XYZ data were pooled with similar XYZ coordinates from the French "Haligure" cruise acquiring Chirp profiles on a dinghy in late August 2009 (P. Henry, pers. comm., 2009). The combined data set is currently processed and will be available in the near future.

7.3.2. Underwater video surveys (ROV, scuba dives)

(A. Kopf, T. Pichler)

Underwater photo and video documentation was severely hindered by large amounts of suspended matter in the water column, mostly caused by a period of gusty winds and rainy thunderstorms in June 2009. As a result, scuba diving operations served mostly to deploy temperature loggers (see next section) and take pictures to document their sites of deployment (see example in Fig. 21). There is also a set of photographs using a digital camera in a transparent pressure housing (Fig. 36a). The scuba diving surveys confirm some of the ROV footage of largely fine-grained drape with variable amounts of pebbles representing the dominant seafloor lithology in the landslide scar as well as along the steeper portion of the headwall. Above the escarpment produced by the 1979 failure, an area with a shallower slope gradient, the amount of pebbles increases significantly and in places appears almost like a "cobblestone pavement". Sizes range from cm- to dm- diameter (Fig. 36b).

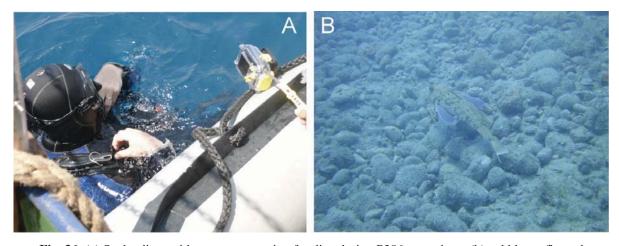


Fig. 36: (a) Scuba diver with camera preparing for dive during P386 operations; (b) pebbly seafloor above the headwall of the 1979 landslide. See fish (appx. 25 cm length) for scale.

Only one long successful dive could be carried out using ROV *Spy* before, as a consequence of bad weather and some mishap in navigation of the vehicle, the device was hit by the propeller of R/V *Poseidon* and could not be repaired on board. This dive, however, was a successful transect starting from some place above the headwall of the NAIL landslide scar resulting from the October 1979 event.

Given the cloudy seawater condition, daylight is limited and headlights were used additionally; hence, some of the still photographs shown here are of limited quality. What is apparent when studying the videos is the fairly flat topography just above the headwall. The seafloor seems to be paved with pebbles of small diameter (<5 – ca. 10 cm across) with no or very little loose sediment in between. No clouds of dispersed material are experienced

when landing the ROV on the seabed (Fig. 37a). Vegetation is limited to the hummocky surface, and no larger seagrass or algae are observed despite operating in the photic zone (15-19 mbsl). When crossing the gently curved headwall while diving down, a drape of soft, loose deposits with occasional pebbles (now larger in size; Fig. 37b) are observed. Both the somewhat steeper gradient and the amount of clasts relative to sediment-covered seafloor continues to depths around 27 mbsl (see Fig. 37c as an example). At around 28 mbsl and below, the seafloor becomes les inclined and uncosolidated sediment prevails. The surface of the sediment shows depressions of small (<5 cm across) to medium size (10±5 cm across) which may represent fluid escape structures. Otherwise, the seafloor is smooth and populated by sparse vegetation (see Fig. 37d-f).

Owing to an increase in wind and swell and the close proximity to the airport of Nice, we had to recover the ROV unexpectedly at this point. Because of the lack of exact positioning of R/V *Poseidon* relative to the vehicle, the ascent of the ROV resulted in a collision with the propeller of the vessel. No other dives were possible during expedition P386 because of the structural damage to the vehicle.

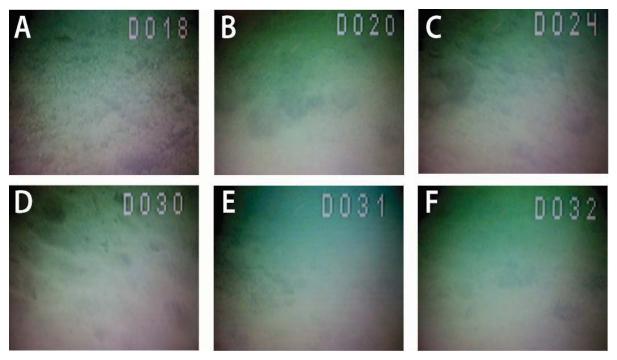


Fig. 37: Panel of ROV still photographs showing typical features from the headwall of the 1979 slide to its central scar in >30 m water depth. Numbers in upper right corner indicate depth in mbsl. (a) Flat area near headwall with a "pavement" of small pebbles (5-10 cm diameter); (b) Larger pebbles (15-20 cm) scattered on soft sediment; (c) inclined slope with pebbles of various size in headwall scar; (d) "pockmark"-like depressions (2-5 cm diameter) in soft, gently dipping sediment at base of headwall; (e) soft sediment with some algae and plant debris; (f) some vegetation and larger (5-10 cm diameter) depressions in the loose sediment (dewatering or degassing structures?). See text.

7.3.3. *In situ* temperature measurements

(A. Kopf, T. Pichler)

In order to test for venting of submarine groundwater discharge (SGD) in a very shallow area close to shore where operation of the R/V *Poseidon* (Fig. 13) is limited, a survey by research divers was carried out from June 26 to 29, 2009 using the vessel *Poseidon III* (Fig. 14). The objective was to deploy temperature probes (this section), to collect pore waters (see Ch. 7.3.8) and to visually survey the seafloor for venting of SGD (see previous Ch. 7.3.2). The purpose of deploying temperature loggers was to test if there would be a measurable change in poor water temperature across several tide cycles. If there had been a measurable change in temperature this would be interpreted as a result of changing the rate of venting as a result of the tide.

The transect was established in approximately east to west direction (110°) across a location where during previous M73/1 operations (Kopf et al., 2008) fresh water was encountered in sediment cores (Kopf et al., 2009). The transect was appx. 120 m long and the whole distance was roped and stakes were driven into the sentiment every 10 m. Temperature loggers were then deployed at transect positions 0, 20, 30, 40, 50, 60, 70, and 120 m (see example in Fig. 21). The corresponding water depths were 15.5, 18.5, 19.9, 21.1, 20.5, 19.9, 22, and 29.5 mbsl. The end point of the transect was marked by a buoy, whose location was N43.64703 and E7.21861 (see map in Fig. 39).

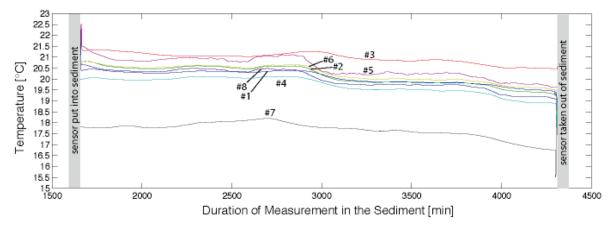


Fig. 38: Temperature data from MTL deployment over almost 48 hrs. along a WNW-ESE transect from above the 1979 landslide headwall to a site close to the long-term piezometer station "Seamonice" (Fig. 39). Note that the westernmost sensor showed by far the lowest T signal, which is interpreted to represent SGD.

The temperature loggers went into the sediment from SE to NW in the following order: 7, 4, 1, 2, 8, 6, 5, and 3. Data of the deployment period, which spanned almost 48 hrs., are presented in Figure 38. It can be seen that a fairly consistent trend between temperature and

water depth is recognised. MTL #7 (29.5 mbsl) shows a significantly lower temperature (appx. 2.2°C) than the suite of other sensors, which scatter between 19 and 21°C (all deployed in the headwall of the NAIL scar). MTL #3 exhibits higher values (20.5-21.5°C), which is attributed to its shallowmost position (15.5 mbsl) above the scar of the 1979 landslide. The measurements indicate indirectly that groundwater seepage is unlikely to occur during the time of the deployment, because the MTLs show a consistent depth-dependent temperature trend. In the shallowmost subseafloor deposits near the headwall of the 1979 landslide scar (MTL #7), temperature is similar to ambient seawater (22-23°C) whereas in the landslide scar in appx. 30 mbsl, it is significantly colder (17-18°C). A SGD signal, which would be lower than ambient seawater during summer, can not unambiguously ruled out for the deeper part of the diving transect (which in fact ends close to the Seamonice station where fresh water is evident; see Ch. 7.3.8 below). However, rhizon sampling from the subseafloor locations equipped with MTLs does not support a groundwater influence (see Ch. 7.3.8 below).

7.3.4. In situ CPT testing

(S. Stegmann, A. Kopf, A. Förster)

A total of 74 FF-CPT drops were conducted over the complete study area (Fig. 39) in water depths between 12 m and 200 m maximum (Table 1). Penetration depth ranged between 1 m and 4 m below the seafloor.

CPT deployments addressed following strategy:

- Completion of the already existing FF-CPT data set of the M73 cruise (Kopf et al., 2008)
- Investigation of the spatial distribution of coarse-grained layers in the level of 40mbsl and its pore pressure regime
- In situ characterisation of sediments running down the western slope
- Comparison between freefall CPT test and CPT profiles pushed with constant velocity (Penfeld CPT, refer to report of PRISME cruise 2007: Sultan et al., 2008)

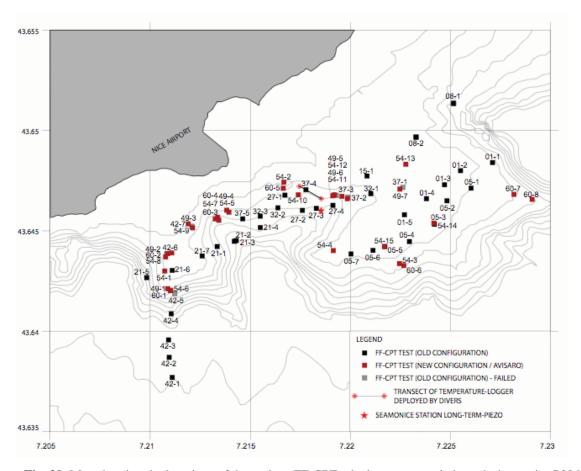


Fig. 39: Map showing the locations of the various FF-CPTu deployments carried out during cruise P386. Some "landmarks" are also shown: "Seamonice" site (star in centre of map), buoy and transect for scuba diving (grey bar).

Owing to the huge number of the FF-CPTu profiles and limited space in the cruise report, only three profiles are illustrated here. They serve to represent the characteristic mechanical differences between the sediments on the plateaus (GeoB13921-06 [Western Plateau]; GeoB13905-03 [Eastern Plateau]) and in the landslide scar (GeoB13927-04).

The sediments on the plateau are characterised by low cone resistance q_t and pore pressure, which is rising after the penetration up to 100 kPa (Fig. 40). Dissipation is never reached in these profiles. Due to the soft und homogeneous composition of the sediments penetration is much higher than in the landslide scar. Maximum penetration depth is 3.70 (estimated, e.g. GeoB13914, -15). For some profiles on the plateau penetration depth is lower, maybe due to the occurrence of coarser-grained layers, which have been detected before with the Penfeld Penetrometer in these deposits (Sultan et al., 2008).

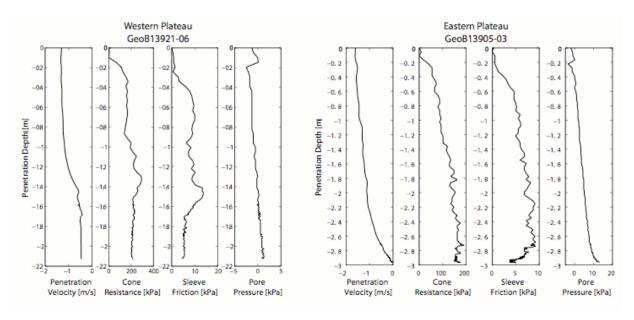


Fig. 40: CPTu records from stations GeoB13921 (W) and –05 (E). See text and map in Fig. 39 for location.

CPTu profiling carried out in the landslide scar delivered penetration depths ranging between 0.9 mbsf and 2.7 mbsf. When running deployments along the 40 mbsl isopach, the coarse-grained, gravel-bearing layer was hit nearly in all CPTu profiles and often terminated the penetration. Cone resistance increased here to up to 1MPa, as illustrated in Figure 41. In contrast to the profiles on both plateaus, pore pressure shows a dilatant behaviour with a sudden drop to negative values.

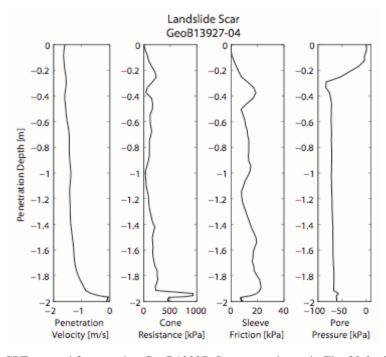


Fig. 41: CPTu record from station GeoB13927. See text and map in Fig. 39 for location.

GeoB 139-	LAT	LON	Date	WD [m]	PTR [m] estimated	PTR [m] calculated	Dissipatio [min]
01-1	43,64845	7,22623	21.06.13	18,20	3,50	3,50	30
01-2	43,64812	7,22572	21.06.13	17,10	3,50	3,20	30
01-3	43,64727	7,22467	21.06.13	16,40	1,50	2,00	30
01-4	43,64662	7,22375	21.06.13	14,80	1,00	1,40	30
01-5	43,64585	7,22278	21.06.13	14,60	3,50	1,60	30
05-1	43,6471	7,22582	22.06.13	20,10	3,50	2,60	30
05-2	43,64653	7,22483	22.06.13	17,20	2,80	3,00	30
05-3	43,64617	7,22508	22.06.13	21,00		2,90	30
05-4	43,6448	7,22307	22.06.13	19,60	>1,5	1,80	30
05-5	43,64418	7,22177	22.06.13	18,20	3,50	2,20	30
05-6	43,64407	7,22115	22.06.13	17,80	3,50	2,20	30
05-7	43,64385	7,22003	22.06.13	15,80	3,50	4,00	30
08-1	43,6515	7,2252	23.06.13	51,00	1,50	1,40	30
08-2	43,6497	7,2232	23.06.13	13,50	2,00	2,20	30
15-1	43,6478	7,2209	25.06.13	13,30			
21-1	43,8442	7,2133	28.06.13	61,00	1,50	1,60	30
21-2	43,8445	7,2142	28.06.13	62,00	1,70	1,80	30
21-3	43,6445	7,2143	28.06.13	64,00	2,00	1,60	30
21-4	43,6451	7,2156	28.06.13	49,20	2,30	1,50	30
21-5	43,6427	7,2101	28.06.13	16,00	2,00	1,30	30
21-6	43,6431	7,2110	28.06.13	13,80	2,00	2,10	30
21-7	43,6438	7,2126	28.06.13	17,00	1,50	1,80	30
27-1	43,6467	7,2173	29.06.13	37,50	2,00	2,30	30
27-2	43,6461	7,2174	29.06.13	44,00	2,70	2,10	30
27-3	43,6461	7,2184	29.06.13	32,50	2,20	2,30	30
27-4	43,6463	7,2192	29.06.13	20,30	1,70	2,00	30
27-5	43,8467	7,2202	29.06.13	16,80	failed	failed	30
32-1	43,6467	7,2210	30.06.13	16,10	1,70	1,30	30
32-2	43,6461	7,2164	30.06.13	39,90	1,50	1,00	30
32-3	43,6458	7,2156	30.06.13	40.80	1,30	1,30	30
37-1	43,6472	7,2225	01.07.13	12.20	1,50	1,50	-
37-2	43,6468	7,2201	01.07.13	18,00			
37-3	43,6468	7,2193	01.07.13	25,90			
37-4	43,6469	7,2176	01.07.13	31,10	2,00	1,00	30
37-5	43,6456	7,21/0	01.07.13	41,10	2,00	1,90	30
					2.20		
42-1	43,6377	7,2111	02.07.13	17,60	2,20	1,40	30
42-2	43,6387	7,2110	02.07.13	14,70	3,60	3,60	30
42-3	43,6397	7,2109	02.07.13	116,00	4.40	2,00	30
42-4	43,6396	7,2109	02.07.13	112,00	1,10	0,90	30
42-5	43,6419	7,2111	02.07.13	62,00			
42-6	43,6438	7,2106	02.07.13	14,00			
42-7	43,6452	7,2120	02.07.13	22,00	4.00		
49-1	43,6421	7,2110	03.07.13	62,00	1,00	not processed	30
49-2	43,6439	7,2109	03.07.13	13,40		not processed	30
49-3	43,6454	7,2119	03.07.13	14,50	2,00	not processed	30
49-4	43,6457	7,2134	03.07.13	40,90	1,50	not processed	30
49-5	43,6468	7,2192	03.07.13	?	2,00	not processed	30
49-6	43,6466	7,2198	03.07.13	?	0,90	not processed	30
49-7	43,6471	7,2225	03.07.13	?	1,40	not processed	30
54-1	43,8429	7,2107	04.07.13	13,80	2,00	not processed	30
54-2	43,6474	7,2167	04.07.13	22,00		not processed	30
54-3	43,6433	7,2226	04.07.13	39,70	3,50	not processed	30
54-4	43,6439	7,2196	04.07.13	14,70	2,50	not processed	30
54-5	43,6461	7,2138	04.07.13	31,60	1,00	not processed	30
54-6	43,6421	7,2111	04.07.13	72,00	1,25	not processed	30
54-7	43,6456	7,2133	04.07.13	42,60	1,90	not processed	30
54-8	43,6437	7,2108	04.07.13	13,30	2,20	not processed	30
54-9	43,6451	7,2122	04.07.13	42,60	0,50	not processed	30
54-10	43,6468	7,2173	04.07.13	?	1,00	not processed	30
54-11	43,6468	7,2191	04.07.13	?	1,20	not processed	30
54-12	43,6467	7,2196	04.07.13	?	1,60	not processed	30
54-13	43,6484	7,2228	04.07.13	?	0,00	not processed	30
54-13	43,6486	7,2230	04.07.13	?	0,00	not processed	30
54-13	43,6487	7,2230	04.07.13	?	0,00	not processed	30
54-14	43,6454	7,2242	04.07.13	?	3,70	not processed	30
54-15	43,8442	7,2217	04.07.13	?	3,70	not processed	30
60-1	43,6421	7,2109	05.07.13	57,00	1,50	not processed	30
60-2	43,6438	7,2111	05.07.13	13,50	1,50	not processed	30
60-3	43,6459	7,2141	05.07.13	33,20	1,80	not processed	30
60-4	43,6456	7,2134	05.07.13	50,00	2,00	not processed	30
60-5	43,6474	7,2167	05.07.13	24,20	0,00	not processed	30
0-5 (repeat)	43,6471	7,2167	05.07.13	28,50	1,50	not processed	30
60-6	43,6432	7,2107	05.07.13	43,70	1,50	not processed	30
60-6	43,6468	7,2227	05.07.13	35,50	9.00	not processed	30
	41.3 (04.00)	1.2261	1 05.07.13	45.50	3,00	TOTAL DESCRIPTION OF THE PROPERTY OF THE PROPE	.90

 Table 1: List of CPTu deployments.

7.3.5. Piezometer deployments

(P. Pelleau, R. Approuial, S. Stegmann, A. Kopf)

7.3.5.1. Operations

Three different types of deployments were carried out with the twp piezometer instruments during cruise P386. The position of the tests and some details regarding the configuration of the probes and deployment procedures are summarised in Table 2.

1. Longterm-installation at the SEAMONICE station using a Piezo v1

The longterm installation at Seamonice provided a continuation of longterm monitoring, which started with a Piezo v1 in November 2006. This instrument was recovered in spring 2009 as one of the connectors was broken and data download by scuba divers became impossible. After maintenance and repair the piezometer was re-deployed during Leg B of cruise P386. The capacity of the power supply is calculated for several years. The connection of the piezometer with the underwater station Seamonice allows an acoustic data transfer without recovery of the piezo.

The deployment at this position took place in several steps (see also Figure 42):

- a) At first the piezometer v1 was deployed at the same position as in the earlier installation (see Fig. 42a, and Sultan et al., 2008). It was lowered to a few meters below sealevel for saturation of the pore pressure lines, and then deployed at full winch speed (1.3-1.5 m/s).
- b) For the release of the piezometer, scuba diving activity was necessary because the acoustic release unit was not designed for weights exceeding 1 ton. As a result, a research diver released the shackle of the instrument (June 29, 2009 at noon).
- c) Once the diver had left, the underwater station (Fig. 42b) was lowered to the seafloor within a distance of 10 meters to the piezometer v1. During this operation the cable, which is used for the connection between the piezometer to the underwater station, was paid out simultaneously. It was crucial to hold both the cable from the deployed piezo v1, but also that of the Seamonice underwater station tight to ensure that Seamonice does not get placed onto the loose cable and possibly damage it. This operation was done using the dinghy with a technician handling the two ends of the cable (Fig. 42c).
- d) After the seafloor unit was acoustically released from the winch cable in appx. 5 m above the seafloor, it settled into its final position for long-term acquisition. The

technician then released the remaining loose cable to the seafloor and the dinghy was recovered.

For more details regarding the positions and configuration of the instrument, refer to Table 2. A simple acoustic communication test (Fig. 42d) between the ship and the Seamonice Piezo v1 was performed immediately after deployment to ensure that the long-term set up is working. This test was successful. In addition, the scuba diver who detached the shackle from the instrument attested by visual inspection and an underwater photograph that the instrument went into the ground vertically to subvertically. The first data set is anticipated to be downloaded by research divers during a cruise in early 2010 (on R/V *L'Europe* led by IFREMER).

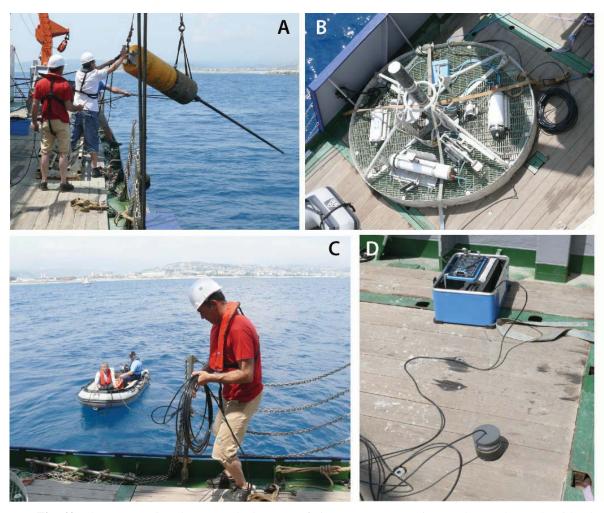


Fig. 42: The "Seamonice" long-tern deployment of piezometer v1: (a) Piezo v1 lowered over the side; (b) the seafloor unit for data acquisition, power supply and communication on deck prior to deployment; (c) dinghy operations to control cable handling during deployment; (d) pinger for acoustic communication test between instrument and ship.

2. Short-term deployments using a Piezo v2

The piezometer v2 (Fig. 43) was deployed on three different positions for periods of 24 hours each (yoyo-mode). Configuration of the lance for the piezo is given in Table 2. Water depth ranged between appx. 10 and 50m. For the duration of the measurement the instrument was decoupled of the wire of the vessel and "moored" with a buoy for recovery. The data were downloaded on board after each test.



Fig. 43: The piezometer v2 ready for deployment offshore Nice airport.

Deployment GeoB13933 took place on June 30, 2009 from 6.00 a.m. onwards. The instrument was first lowered above the planned position of deployment on the non-failed portion of the slope west of the NAIL 1979 scar. After 20 mins. on the wire in 10 mbsl for saturation of the pore pressure tubing, the device was veered at a rate of 1 m/s until it penetrated tha sediment. It was then detached from the winch cable, and a surface buoy was mounted for the 24hr-dissipation test (Fig. 44). The position corresponds to that of a gravity core (station GeoB13946; see below and station list in Appendix 10.1).

On July 1st, 2009 GeoB13933 was ended by recovering the instrument (appx. 6.00 a.m.) and downloading the data on deck. After about an hour, while R/V Poseidon had drifted to the next position at the northern rim of the 1979 NAIL scar, the piezometer was deployed again for 24 hrs. following the above procedure (station GeoB13938). The position corresponds to that of gravity core GeoB13953 (see below and station list in Appendix 10.1).

The piezometer was again recovered on July 2nd, 2009 (6.00 a.m.), followed by successful data download on deck. It was noted that penetration was highly efficient since fine-grained sediment was found both on top of the base plate as well as various places on the weight set overlying the base plate (see Fig. 23 above). The instrument was redeployed at station GeoB13945 in the eastern, non-failed slope in the position where a gravity core was taken earlier (GeoB13919; see "Lithostratigraphy" section below and Appendix 10.1). A day later, the piezometer was recovered with a slight bent as a consequence of having pulled it out of the seafloor at an angle. Data were recovered successfully and the instrument was thereafter prepared for the long-term deployment (see next paragraph).



Fig. 44: Surface buoy of piezometer v2 after deployment offshore Nice airport.

	Station GeoB-	Latitude	Longitude	Date	Test		WD [m]	Duration [h]	Sensors	Sampling Rate [s]	Weight in air [t]
	13931	43,6457	7,2178	29.06.2009	Piezo v1	long-term	40,3	still installed	P1 (0.50m), P2 (1.50m), P3 (2.25m), P4 (3.00m), P5 (3.75m)	300	1,1
L	13933	43,6426	7,2110	30.06.2009	Piezo v2	yoyo	14,4	24	P1 (4.75m), P2 (0.80m), P3 (2.40m), P4 (1.60m), P5 (3.20m)	1	1,2
	13938	43,6414	7,2123	01.07.2009	Piezo v2	yoyo	13,2	23	P1 (0.80m), P2 (1.60m), P3 (2.40m), P4 (3.20m), P5 (4.75m)	1	1,2
L	13945	43,6471	7,2166	02.07.2009	Piezo v2	yoyo	34,0	23	P1 (0.80m), P2 (1.60m), P3 (2.40m), P4 (3.20m), P5 (4.75m)	1	1,2
	13959	43,6440	7,2194	04.07.2009	Piezo v2	long-term	15,2	still installed	P1 (0.80m), P2 (1.60m), P3 (2.40m), P4 (3.20m), P5 (4.75m), P6 (5.50m)	300	0,25

Table 2: Information regarding the deployment and configuration of the various piezometer tests.

3. Longterm-installation using a Piezo v2

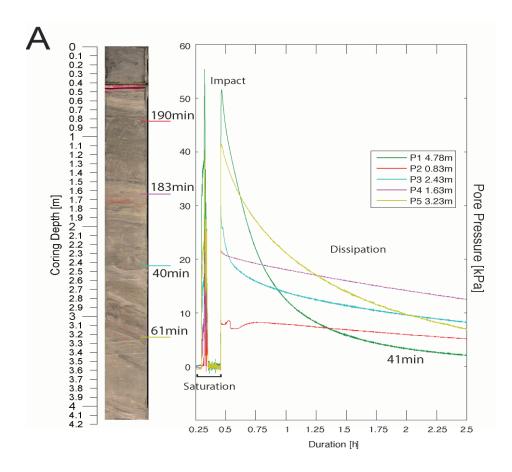
After the yoyo-tests the piezometer v2 was deployed for a longterm-test at station GeoB13959. This location has been chosen to be identical to an earlier deployment by IFREMER during the *PRISME* cruise (Sultan et al., 2008). The capicity of the power supply

was calculated for 2 years and sampling was set to a period of 5 minutes. Sampling was synchronised with the Seamonice piezometer v1 to get a comprehensive data set from the pair of instruments in the stable (v2) and failed (v1 plus Seamonice) portion of the Nice slope. In contrast to the piezo v1, the new instrument cannot be visited by scuba divers for data download, but has to be recovered.

On July 4th, 2009, the piezometer v2 was prepared for the long-term monitoring task tentatively set up for 24 months. Preparation included the replacement of the bent section of the instrument, its elongation by another 75 cm-long segment plus one piezomodule, and attachment of a small buoyant device for it to be recognised by research divers for future recovery. After the obligatory saturation of the pore pressure tubing in the water column (9:30 a.m.), the device was deployed at 1 m/s winch speed. The weight set was then released acoustically and got recovered back on deck at 9:44 a.m. During the upcoming cruise led by IFREMER in early 2010 (on R/V *L'Europe*), divers will inspect the piezo v2 when downloading data from the v1 instrument.

7.3.5.2. Preliminary results

The first "yo-yo" style piezometer deployment targeted the non-failed portion of the Nice slope west of the NAIL scar. The measurement at five levels recorded the artificial pore pressure spike during impact followed by 24hr-dissipation curves (GeoB13933; Fig. 45). In the same position, core GeoB13946 recovered sediment down to 4.15 mbsf. The data illustrate that depending on the lithology encountered at each depth level, both the magnitude of the initial pressure spike as well as the t50 values from pore pressure dissipation (see Davies et al., 1991) vary significantly. In the shallow portion where clayrich deposits dominate (level P2, P4; Fig. 45a), the decay of the initial pore pressure value is slow and half of the peak is reached only after 183-190 mins. In contrast, the somewhat siltier portion of the slope (below 2 mbsf) allowed fluid to be displaced during the insertion of the piezometer, and equally faster decay of the pore pressure pulse. As a consequence, t₅₀ values range between 40 and 61 mins. (Fig. 45a) and decrease along shallower gradients. All pore pressure ports reach a "quasi-steady state" with a plateau after 8 and 11 hrs. after deployment and generally remain at this level for the remainder of the measurement period. At the deeper levels, those "background values" are very close to hydrostatic pressure at the respective depth (P1, P3, P5) whereas the shallower, less permeable deposits at level P2 and P4 show pore pressures of 2-4 kPa in excess of P_{hyd} (Fig. 45b).



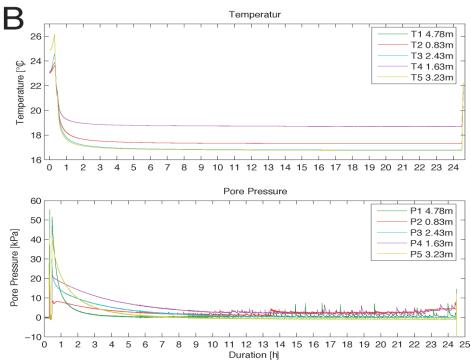


Fig. 45: Data from piezometer v2 deployment GeoB13933 offshore Nice airport. (a) Data during insertion and t₅₀ values derived from them. The locations of the individual piezo modules at depth is plotted on a gravity core photograph at the same site. (b) Long term behaviour of T and pore P at this station. See text.

There are two rather interesting phenomena observed in the second half of the 24hr. deployment. First, all pressure ports become increasingly noisy after appx. 7 hrs. and remain this way throughout. There does not seem to be a difference in noise level between the more permeable layers compared to the upper clayey levels. The spikes, sometimes a few kPa in magnitude, are not necessarily synchronous, so they can hardly be explained by tidal variations (the latter being rather small in the Baye des Anges anyway). At this level, we can only speculate about gas migration as a possible candidate to explain these patterns. The second interesting phenomenon was observed only in the slightly overpressured ports P2 and P4. After appx. 22.5 hrs in the deployment, the pore P level started to gently rise to levels of 5-7 kPa excess pore pressure (Fig. 45b). During the same period, the more permeable layers did not transmit any pressure change to the instrument. It is unclear if this late increase is approaching the "true ambient" background pressures, or whether external forcing in the shallowmost succession has to be held responsible.

For the second "yo-yo" deployment of the v2 instrument, the northern headwall area of the NAIL scar was chosen. The piezometer test (GeoB13938) is complemented by a gravity core taken in the same location (GeoB13953; see map in Fig. 48 below). Data from the piezotest are shown in Fig 46. Given that the core recovered was somewhat short of 3 m length, the lowermost piezomodules (P4, P5) are unconstrained regarding the lithology they were inserted into. The plot of the results vs. 24 hrs. of deployment reveals two distinct pore pressure responses. Ports P1-3 show the anticipated strong increase in pore pressure upon penetration of the instrument, which is followed by an exponential decay towards a background value. Calculated t₅₀ values range between 40 and 61 mins. (Fig. 46a). In contrast, ports P4 and P5 show a less dramatic increase during insertion of the piezometer, which becomes less and less strong and tapers off towards maximum 10-30 mins., not seconds. After having reached the maximum value, pore pressure slowly decreases with time, eventually approaching ambient values. The majority of the pore pressure ports (namely P1, P3, P4) reach hydrostatic pressure over the course of the experiment. Transducer P2 at 1.63 mbsf also approaches hydrostatic levels, however, but after 10 hrs. within the test the signal becomes noisy and shows a subtle increase in pore pressure (Fig. 46b). This saw tooth pattern, which was also observed in deployment GeoB13933 (see above; Fig. 45b), becomes more accentuated after appx. 19 hrs.; the increase in noise is accompanied by an overall increase in pore pressure to values of 10±2 kPa in excess of Phyd (Fig. 46b). An exception to the overall behaviour is the deepest transducer P5 (Fig. 46b).

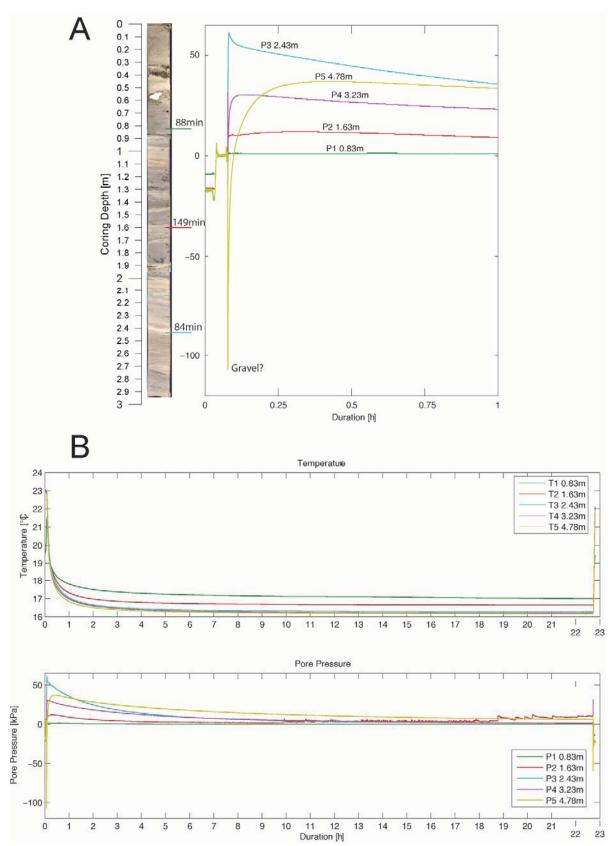


Fig. 46: Data from piezometer v2 deployment GeoB13938 offshore Nice airport. (a) Data during insertion and t₅₀ values derived from them. The locations of the individual piezo modules at depth is plotted on a gravity core photograph at the same site. (b) Long term behaviour of T and pore P at this station. See text.

After having reached the maximum value, pore pressure levels off towards an excess pore pressure value of 6 kPa. Given that the gravity corer did not penetrate that far, it is unclear what lithology is related to the subtle overpressures. A careful comparison to quasi-static Penfeld CPT tests (Sultan et al., 2008) is required as part of the post-cruise research. In summary, core GeoB13953 was the most diverse of all gravity cores recovered during cruise P386 (see detailed description in section 7.3.6 on "Lithostratigraphy" below), so that it is not surprising that the pore pressure response is also found rather variable.

The third v2 deployment was positioned on the slope east of the NAIL scar. It mirrors the position of an earlier piezometer test in 2007 (PZ21-2; see Sultan et al., 2008) and is complemented by gravity core GeoB13919 (see 7.3.6 and Appendix 10.3). The piezometer test was also a pilot study for the long-term deployment of the v2 instrument, which was performed in exactly the same position later during the cruise (see station GeoB13959 in section 7.3.5.1 above; also refer to station list in Aappendix 10.1).

Similar to test GeoB13933 at its western counterpart, all 5 piezomodules of test GeoB13945 east of NAIL show the typical strong increase in pore pressure during insertion of the probe. This spike is followed by a moderately strong (P3, P4) to strong (P1, P2, P5) decay (Fig. 47a), which results in t₅₀ values of less than an hour (48 mins. For the upper, silt-dominated protion) and between 89 and 233 mins. for the clay-dominated section below 2 mbsf (see Fig. 47a, and Appendix 10.3). When regarding the entire period of the deployment, both T and pore P show a rapid exponential decrease towards ambient values. For the temperature, this is 15.5°C whereas the pore pressure is very close to hydrostatic values (maybe a little higher for P5 at the deepest level). Interestingly, this transducer also shows the noisiest record from appx. 11.5 hrs. into the deployment onwards, showing spikes that exceed the background decay curve by 5-12 kPa in places. Like for the variations depicted in the earlier deployments, it is not easy to explain those fluctuations. Despite the fact that free gas is inferred from geophysical data and observations on cores (see Sultan et al., 2008, in press, and section 7.3.6 below), it is impossible to assess with certainty that gas migration or expansion can be held responsible for pressures which in places are close to lithostatic values. Post cruise experimental work on the corresponding cores as well as the long-term data records from the two peizometers deployed for months to years will hopefully help explain some of these preliminary observations.

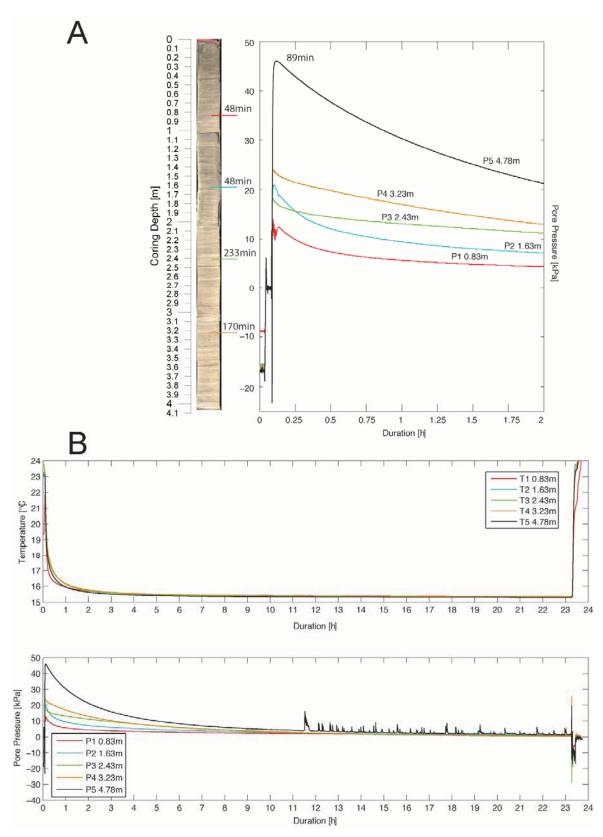


Fig. 47: Data from piezometer v2 deployment GeoB13945 offshore Nice airport. (a) Data during insertion and t₅₀ values derived from them. The locations of the individual piezo modules at depth is plotted on a gravity core photograph at the same site. (b) Long term behaviour of T and pore P at this station. See text.

7.3.6. Gravity coring and sediment description

(A. Förster, T. Fleischmann, K., Weber, S. Stegmann, A. Kopf)

During cruise P386, we took 23 gravity cores as well as a pair of "bobcores", the majority of which were recovered in the NAIL area. Gravity cores GeoB13910, -11, -12, -13 and -14 complemented earlier coring in the deeper (>1500 mbsl) research on the so-called "Western slide complex" adjacent to the Var Canyon (see M73/1 cruise report by Kopf et al., 2008, and Förster et al., 2009). The cores were left closed for shore-based geotechnical testing. For exact location of these cores, please refer to the station list in Appendix 10.1 below. An initial sedimentological description from post-cruise work is given in Appendix 10.2.

The two "bobcores" served as a proof-of-concept with the recently developed lightweight coring system. The recovery was found low (<50 cm) despite repeated use of the falling weight and the predominantly fine-grained material. Given that gravity cores were taken nearby, the two bobcores were also left unsplit and got archived. For exact location, please refer to the station list in Appendix 10.1 below.

The lithological description focuses entirely on the NAIL slide scar and adjacent slope. The wealth of cores is shown in map view in Figure 48. Broadly speaking, they can be separated into three groups: (i) cores in the headwall and scar area of the 1979 event; (ii) cores in the non-failed slope E, N and W of the NAIL scar, and (iii) cores recovered in somewhat deeper water where the slope has also been stable to date. All these cores were taken in water depths less than 100 mbsl except for core GeoB13918; for exact depths refer to station list in Appendix 10.1 below. The grouping of the cores is as follows:

Group I: Cores GeoB13925, -29, -30, -34, -39, -40, -53, <u>-63</u> and <u>-64</u>.

Group II: Cores GeoB13928, -46 (both W' of NAIL), and cores -19, <u>-20</u>, -26, <u>-35</u>, <u>-36</u>, <u>-41</u>, -42, -51, and -52 (E' of NAIL).

Group III: Core GeoB13947 (W' of NAIL), and cores <u>-56</u>, <u>-57</u>, <u>-58</u> and <u>-64</u> (E' of NAIL). In addition, cores GeoB13918 and <u>-48</u> were taken south in and adjacent to a block of sediment which presumably slid to its present position in October 1979.

Note that the cores which are <u>underlined</u> above were not opened during cruise P386 so that core description does not exist.

Regardless of the geographical distinction into several groups, a group of gravity cores is also to be separated because the represent the shallowmost portion of nine locations where IODP drill sites have been proposed (IODP proposal 748-full by Stegmann et al., 2009).

These cores are GeoB13928 (NA-01), -29 (NA-02), -39 (NA-03), -40 (NA-04), -46 (NA-05), -47 (NA-06), -52 (NA-07), -19 (NA-08) and -18 (NA-09). Most of the NAIL cores plot in the vicinity of Nice airport, however, GeoB13918 (i.e. NA-08) lies some 2-3 km south of the 1979 scar in appx. 150 m water depth (see Fig. 12; and also Stegmann et al. [2009] and station list in Appendix 10.1).

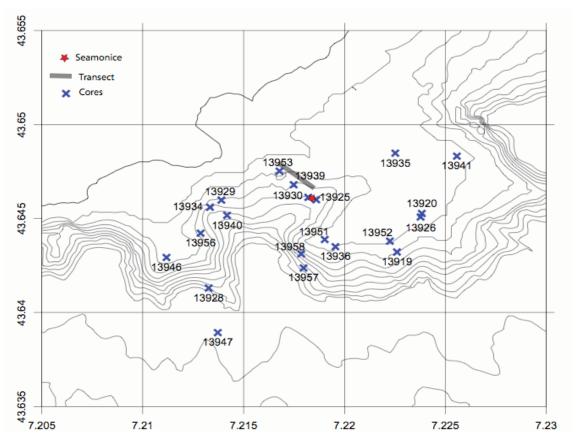


Fig. 48: Map of the Nice airport region showing the locations of the majority of the gravity cores taken during P386. Note that cores GeoB13918 into a slid block of the 1979 event and –48 (a little north of –18) are located south of the map. The grey bar marks the scuba diving transect and the red star represents the "Seamonice" long-term piezometer.

When regarding the sediment recovered in the different areas, there is a clear separation between the groups. They are described one by one summarizing the main caharcteristic sedimentological and structural features. Detailed lithological columns with descriptions, grain size classification, and some physical properties are given in Appendix 10.2 below. Scanned images of the entire core s well as results from MSCL logging of the cores (split as well as unsplit ones) is found in the "Physical Properties" section below (Ch. 7.3.7) as well as on the DVD in the back sleeve of this volume (Appendix 10.3, only available electronically).

Group I cores in the 1979 landslide scar

The headwall area and steeply inclined scar of the 1979 failure were among the main goals of coring and sampling during cruise P386. As a result, nine gravity cores targeted this zone in water depths between approximately 20 and 40 mbsl (Fig. 48). Core recovery was highly variable because fine-grained, soft sediments are interbedded with gravel layers.

The major lithologies in the cores recovered were clays with variable amounts of silt. These sediments occasionally show darker, organic-rich layers (mm to a few cm) or lenses, but are usually medium to light grey to yellowish. Shell fragments, bioturbation, wood pieces or sand patches are also observed. Some of the clay-rich intervals show distinct fractures and conduits which were interpreted as evidence for fluid escape (see core geoB13940 below 250 cm bsl; refer also to lithological descriptions, Appendix 10.3). Apart from core GeoB13940, each of the headwall cores also showed at least one layer of gravel (e.g. Fig. 49b). The size of the generally well-rounded pebbles varied from 1-10 cm across, embedded in a yellowish, sandy and silty matrix. Examples from cores GeoB13929, -30, and -34 are given in Fig. 49a-c and show the disturbed nature of some of the layers. The suite of observations include normal faulting, slumping, erosive contacts between layers, and entrainment of silt/sand into finer areas. For the full set of observations, refer to litholog diagrams in Appendix 10.2.

A extraordinary core was recovered at station GeoB13953 in the northern headwall (Fig. 48). It was taken right at the northern rim of the NAIL headwall in 19.7 m water depth. In the upper portion (0-140 cm bsf), it comprises predominantly yellowish to beige sand with pebbles and shell fragments, rarely interbedded with dark grey silt. The interval between 30 and 60 cm shows rapidly buried seagrass in large quantities (Fig. 50a) and further contains a several cm long plastic fragment, including the threaded opening of what appears to be a food container. The date of manufacturing of this artifact, 09/1976, is embossed into the fragment (Fig. 50b). We tentatively interpret this deposit as directly related to the 1979 catastrophic event, either as a landslide deposit burying seagrass or as a tsunami deposit in very shallow water. The plastic container may have served as a fisherman's buoy at the time, or was simply rubbish floating around. Below this rather unusual layer, silty and sandy layers are interbedded with finer-grained starta. In various places, thin, red clay bands and layers are observed. Clay lenses are also found and may represent remobilized material

associated with the landslide. Below 170 cm bsf, the sediment comprises almost pure clay (for details, see Appendix 10.2).

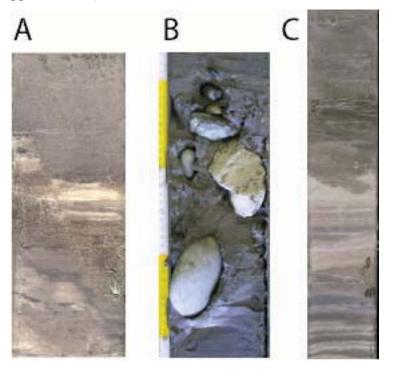


Fig. 49: Cores from the headwall area of the Nice airport slide. (a) GeoB13929, 147-162 cm with clearly developed normal fault in clay-silt interbeds; (b) GeoB13930, 47-78cm showing coarse gravel deposits in a yellowish to beide clay matrix; and (c) GeoB13934, 103-155 cm with tilted, sometimes erosive contacts and evidence for slumping. See text.

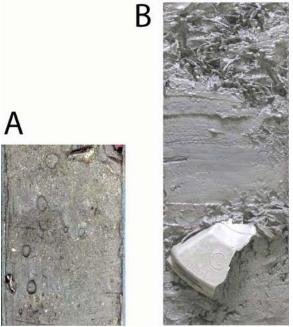


Fig. 50: CoreGeoB13953 from the headwall area of the Nice airport slide. (a) coarse-grained sand with pebbles (8-20 cm bsf); (b) Inferred "tsunami" or "syn-depositional landslide" deposit with the plastic bottle fragment with date (32-62 cm bsf).

Group II cores at the non-failed slope adjacent to the 1979 landslide scar

A total of eleven gravity cores were taken in the non-failed slope east (9) and west (2) of the 1979 NAIL scar (Fig. 48). Core recovery was generally good because of the poorly to normally consolidated sediments. The sediments comprise fine-grained (clay, silt) deposits of poor to normal consolidation. In several cores, underconsolidation and evidence for the presence of gas is observed. This is manifested by gas pockets and fluid escape structures as well as gas expension cracks in the sediment (see Fig. 51).

The cores taken in the non-failed portion of the slope are rarely west of the slide scar (GeoB13928, -46), mostly because the "plateau there is rather narrow (Fig. 48). The majority was taken east of NAIL, namely cores GeoB13919, -20, -26, -35, -36, -41, -42, -51, and -52. Since one of the key goals of the expedition is to characterize some geotechnical properties, half of the cores in question were left unsplit and were shipped back to MARUM, Univ. Bremen. MSCL data of the unsplit cores are available in Appendix 10.3 (only on CD in back pocket).

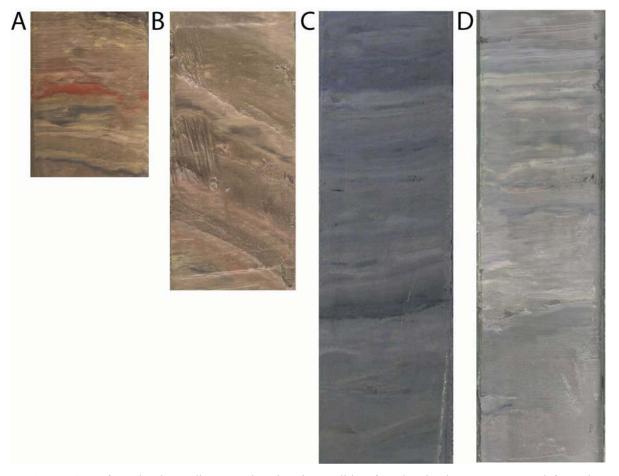


Fig. 51: Cores from the slope adjacent to the Nice airport slide, often showing homogeneous, undeformed clayey sediments with occasional silt bands. Examples given are from intervals (a) GeoB13946, 36-49 cm, (b) GeoB13946, 69-89 cm, (c) GeoB13919, 328-365 cm, and (d) GeoB13952, 157-179cm. For location of cores, see Fig. 48.

The majority of the cores taken on the shallow-dipping slope east and west of the NAIL scar are characterized by homogeneous clay of brownish to ellowish and grayish colours. Colour banding and interbedding with silt is also common (e.g. Fig. 51). Rarely, irregular sedimentation patterns such as tilted surfaces or clay clasts and lenses are found. Given the extremely fine-grained matrix of the dominant lithology, permeability is low (Weber & Kopf, unpubl. data) and fluids are confined in these sediments at in situ conditions. Once the core is recovered and opened, fluids such as microbial methane expand and cause porous (mousse-like) textures and small (1-4 mm diameter) gas pockets in the otherwise undisturbed matrix (Fig. 52a, b). Aqueous fluid also migrated through the fine-grained sediment, causing entrainment of some matrix material during the ascent. Evidence for dewatering and fluidisation is found as near-vertical channels and conduits (e.g. Fig. 52c).

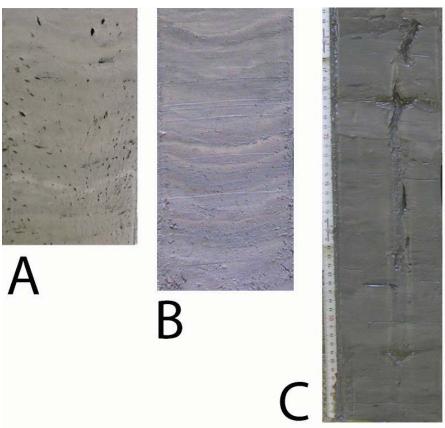


Fig. 52: Cores from the slope adjacent to the Nice airport slide, dominated by clayey sediments with abundant evidence for aqueous and gaseous fluids. Examples given are from intervals (a) disruption from gas expansion (b). Examples given are (a) GeoB13928, 286-300cm, (b) GeoB13946, 372-394 cm, (c) GeoB13926, 141-177 cm. For location of cores, see Fig. 48.

Group III cores at the somewhat deeper slope

This group can be divided into two subgroups. The first subgroup comprises cores GeoB13947, -56, -57, -58 and -62. It shows fairly homogeneous, fine-grained sediments which got recovered in cores of 4.59 to 5.39 m total length. These cores have mostly been

taken for post-cruise geotechnical studies on slope stability under different conditions. They were not opened except for a few sections of cores GeoB13957 and -58. Most interestingly, section GeoB13957-3 has several conduits of 5 mm to 12 mm diameter and several tens of cm in length (Fig. 53a, b). They are subvertical and intersect a clayey interval which is under- and overlain by silty, more permeable units. One of the conduits connects the two silt layers and may have been a result of fluidisation and release of overpressure by causing a predecessor to hydraulic failure (see Mörz et al., 2007).

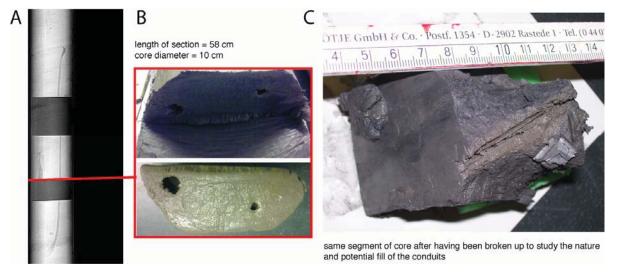


Fig. 53: Core GeoB13957: (a) X-ray scan of a temporarily opened whole core cut layer-parallel in order to show the diameter of the fluid conduits (b); one segment was further broken up (c). See text.

In contrast, the second subgroup contains two cores located in significantly deeper water some 2-3 km south of the 1979 scar where the mass flow migrated downslope towards the Var Canyon. One of these cores, GeoB13918, equals the proposed drill site NA-09 into a portion of partly stratified sediment which is inferred a slid block. Three meters of silty clay with darker layers, fine- to medium sand interbeds and lenses, and occasional shell fragments were recovered. In the shallowmost portion, the clay layers are water-rich and free of internal structures. Deeper in the predominantly homogeneous core, areas rich in organic matter and irregular contacts occur (Fig. 54). Lenses of clay or silt are abundant between 100-200 cm bsl, however, it is impossible to decide whether they represent rip-up clasts. At the base of the core (290-300 cm bsl) is a gravel layer in which the gravity corer lost its momentum and did not penetrate further. The pebbles are well rounded, composed of quartzite and claystone/siltstone, and measure a few cm in diameter. Although the presence of these pebbly layers is generally associated with landsliding in the Nice area (see Group I above where each core has at least one pebbly layer, often the area where fresh water is measured (see "Geochemistry" section below), it is not clear at this stage whether the

material is unambiguously part of a remobilized block formerly located in the upper slope. The second of these cores, GeoB13948, is located somewhat upslope at the northern border of the inferred slid block. From seismic reflection data, the location may have "Upper Pliocene" substratum (*sensu* Savoye et al., 1993) cropping out at the seafloor. Since this stratum is an assumed potential failure horizon, we tried to sample it for geotechnical testing. However, the core has not been split and described yet.



Fig. 54: (a) Photograph of GeoB13918, interval 110-155 cm bsl, showing light to dark grey, clayey and silty deposits with irregular surfaces and erosive contacts. Top of core is left. See text.

7.3.7. Physical properties

(A. Förster, T. Fleischmann, S. Stegmann, K. Weber, A. Kopf)

Key physical properties for the landslide study offshore Nice Airport were measured on the fresh, split core (namely shear strength) and also shore based in a non-destructive manner (density, porosity, magnetic susceptibility and p-wave velocity using the MSCL). A number of other properties was further determined on discrete samples for specific projects and is not reported here. Instead, the following two subchapters focus on the shear strength and MSCL data. All shear strength data from fall cone experiments and vane shear tests can be found in Appendix 10.2; all MSCL data are given electronically in Appendix 10.3 (CD-ROM only).

Shear strength

As for the core description, the shear strength data are presented as regional groups of cores. Group I in the headwall scar of NAIL often shows a gradual increase in strength, with values <10 kPa in the upper meter and then higher values (appx. 10-25 kPa for vane shear, and somewhat higher values for fall cone tests) below. Although layers rich in pebbles were avoided, maximum strength results may reach values >200 kPa (e.g. GeoB13925). We interpret these data as exceptions, possibly because a shell fragment or indurated clast was hit during the measurement. Silty and sandy horizons often show twice the strength as their

finer-grained counterparts. Areas with unusual deposits or other anomalies (e.g. the seagrass in GeoB13853) were avoided for strength measurement. Group II in the stable portion of the slope generally shows lower strength, because the majority of the cores recovered mainly clayey muds. Even long (i.e. >4 m) cores rarely exceed strengths of 10 kPa. All cores show a subtle increase in strength with depth, neatly mirroring incipient settling and compaction. Even fall cone data, which tend to accentuate shear strength trends because of the dynamic mode of measurement, plot below 10 kPa except for individual measurements (see GeoB13919 and -46, lower part) or core GeoB13926, where silt is more abundant. Group III cores remained largely unsplit, so that only GeoB13918 delivers shear strength data at this stage. Despite the fact that the main lithology is clay, the majority of the vane shear data ranges >10 kPa and the fall cone data largely plotting between 10 and 35 kPa. We tentatively interpret this observation as a result of the core being taken at position NA-09 (NAIL IODP drilling proposal by Stegmann et al., 2009). This site represents a block of slid material and, as a consequence, may have been modified during emplacement so that more indurated material is now in the shallow subsurface.

MSCL

From the suite of MSCL data, only density (and, derived from that, fractional porosity) plus some susceptibility information will be regarded. For all other data, refer to CD-ROM in back pocket (Appendix 10.3).

Group I cores in the NAIL headwall show average densitie sranging from appx. 1.8 g/cm³ (GeoB13925, -53) to 2.05 g/cm³ (GeoB13934). In core GeoB13939, very soft mud (1.7 g/cm³) overlies significantly denser sediment (2.15 g/cm³). We interptret these variations as evidence for unroofing during the 1979 event, i.e. in places where densities are high the soft overburden got remobilized and transported downslope. Fractional porosity mirrors these trend in opposite direction, and magnetic susceptibility values often are high in sections where density is low owing to clay mineral-rich mud.

In Group II cores on the non-failed slope E and W of NAIL, many cores show low bulk density values of 1.75 ± 0.05 g/cm³. Examples include cores GeoB13926, -28, -41, -51 and – 52. Other cores show somewhat higher densities and porosities of appx. 45-50% (GeoB13920, -46). Core GeoB13919 shows exceptionally high densities (and resulting porosities of appx. 40%), which is in contradiction with the observations from core description and shear strength measurement (see above and Appendix 10.2). It appears that these soft (shear strength < 10 kPa) silty clays contain high contents of dark, fibrous organic

matter, which may be responsible for the higher density. Magnetic susceptibility and p-wave velocity do not show any unusual excursion in this core, so that post-cruise study on discrete samples may be required to confirm the density values of 2.1 g/cm³ on average.

Cores of Group III taken downslope of the landslide scar show consistent physical properties. Since the majority of these cores remained unopened, MSCL data are the only information about the material recovered at this point. In general, density shows a gradual increase with depth, starting at ca. 1.7 g/cm^3 at the seafloor and gently increasing to up to 1.9 g/cm^3 at the terminal depth of the core. Some cores appear extremely rich in fluid in the shallow sub-seafloor, starting with densities of 1.5 g/cm^3 and porosities of ca. 70%. Magnetic susceptibility is generally below 10, however, there are two exceptions whee values are fairly high: Cores GeoB13947 (0.6 - 3.3. mbsf) and -57 (0.7 - 2.2. mbsf). Since the latter core was opened and examined because of its fluid channels (see above and Figs. 53, 54b).

For a more comprehensive overview, see Appendices 10.2 and 10.3 below.

7.3.8. Pore water geochemistry

(T. Pichler, S. Pape, S. Hammerschmidt, R. Price, M. Seydel)

This chapter is split in two. The first (larger) section will tackle the pore water geochemistry of GeoB cores, while the second half will introduce the results from pore waters extracted from the shallowmost subseafloor using syringes.

In total 225 pore water samples were collected from 13 gravity cores and analysed for physical and chemical parameters. A detailed listing of these parameters is given in Tables X1 and X2. Salinity ranged from that of fresh to seawater, i.e., <100 mg/L to 35 000 mg/L. Seven sediment cores (13925, -29, -30, -34, -39, -40, -53) showed a clear impact of freshwater, while cores 13919, -26, -28, -46, and 13952 had the pore water composition which indicated only seawater. Those cores where a fresh water component was observed were generally located in the center of our study site (Fig. 55). Fresh water, although present in the cores, did not seem to exit the sediments in the form of submarine groundwater discharge (SGD). All the pore water profiles changed to more or less seawater composition within the top 50 to 100 cm (Fig. 56). This was also confirmed by the pore waters, which were collected by scuba (see below). In general, the pore water profiles can be divided

following Schlüter et al. (2004):

- profiles which did not show any fresh water impact (linear profiles of Cl and Na);
- profiles where fresh water is present at depth and where curvature of the Cl and Na data series indicates that some advective transport was present;
- profiles where fresh water is present at depth and where the linearity of the Cl and Na data series indicates that diffusive transport was present.

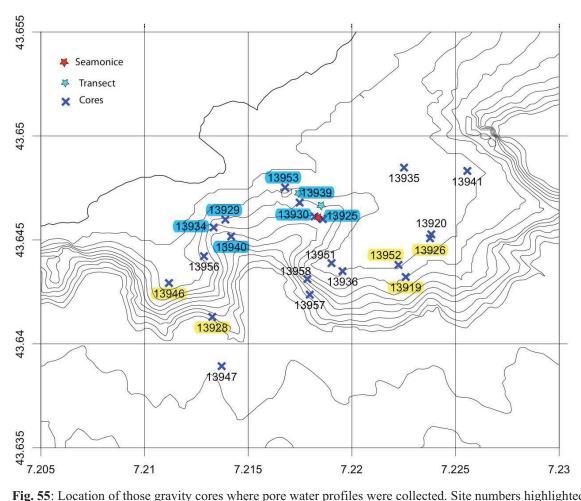


Fig. 55: Location of those gravity cores where pore water profiles were collected. Site numbers highlighted in "yellow" indicate pore water profiles with seawater salinities and linear Cl and Na data series. Site numbers highlighted in "blue" indicate pore water profiles, which contained a fresh water component.

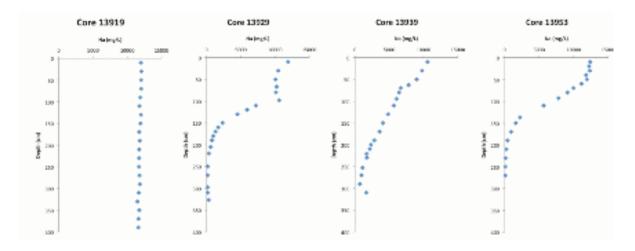


Fig. 56: Pore water profiles of sodium (Na) concentration in mg/L for cores 13919, -29, -39, and -53.

The pore water profiles 13952, 13953 and 13939 were examined closer, because they are examples for (a) a seawater water dominated profile, (b) a profile with advective transport and (c) a profile with diffusive transport, respectively. These profiles are presented in Figures 57, 58 and 59.

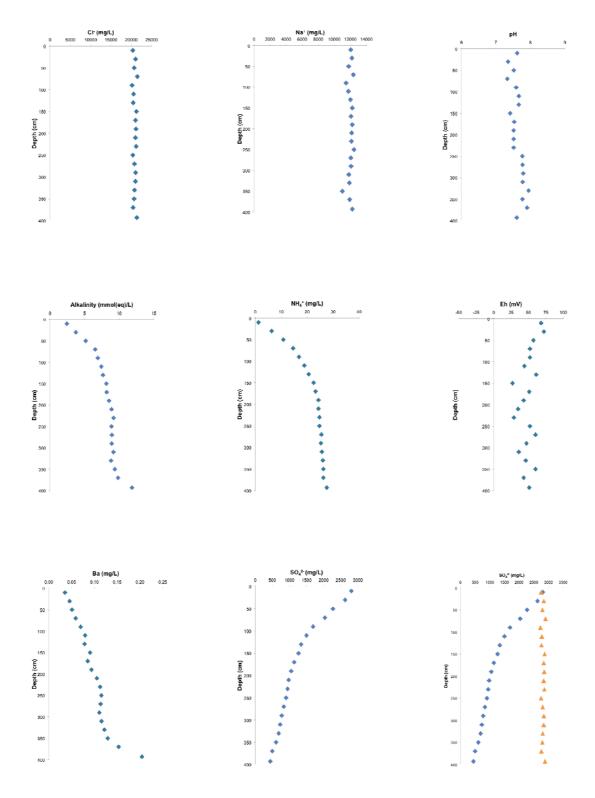


Fig. 57: Pore water profiles for Cl, Na, pH, Alk, NH₄, Eh, Ba, SO₄ and SO₄ calculated for gravity core 13952. The yellow triangles in the profile in lower right corner represent SO₄ values, which were calculated based on the Cl/SO₄ ratio in seawater.

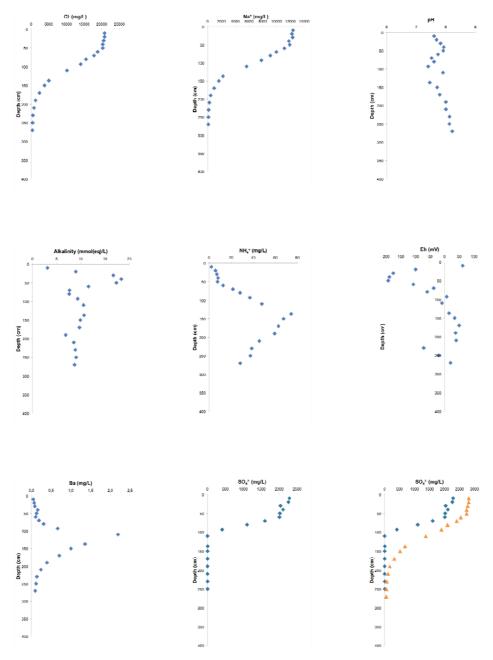


Fig. 58: Pore water profiles for Cl, Na, pH, Alk, NH₄, Eh, Ba, SO₄ and SO₄ calculated for gravity core 13953. The yellow triangles in the profile in lower right corner represent SO₄ values which were calculated based on the Cl/SO₄ ratio.

Pore water profile 13952 was chosen as a representative of a profile dominated exclusively by seawater. Here, all Cl and Na values show no variation with depth and values were around 20,000 mg/L for Cl and 12,000 mg/L for Na, which are representative of Mediterranean seawater. pH values showed a little variation generally ranging between 7.4 and 7.6. These values are slightly lower than expected for seawater. Alkalinity and ammonia increase with depth to several times seawater values. Eh shows some variation, which is likely to two the uncertainty of the measurement, however, values are always positive. Barium increases with depth from the seawater value to about 0.2 mg/L. Sulfate declines

from almost 3000 mg/L (a value expected for seawater) to about 500 mg/L since Cl and Na to not show this decrease in concentration it is expected that sulfate does not behave inert. The likely explanation for the decrease in sulfate is microbial reduction to sulfide (e.g., Schulz et al., 1994; Winfrey et al., 1981). The amount of sulfide production can be estimated based on the Cl concentrations. Assuming that the Cl/SO₄ ratio in seawater is constant, the concentration of sulfate can be calculated using the measured Cl value. The comparison between the measured and calculated sulfate profiles can be seen in Fig. 57. Sulfate reduction increases constantly with depth.

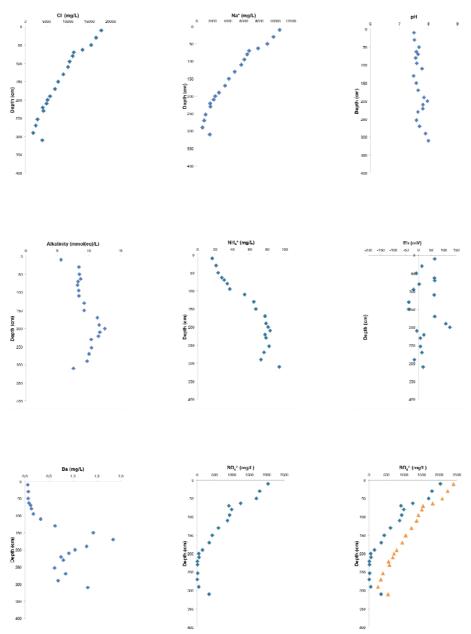


Fig. 59: Pore water profiles for Cl, Na, pH, Alk, NH₄, Eh, Ba, SO₄ and SO₄ calculated for gravity core 13953. The yellow triangles in the profile in lower right corner represent SO₄ values which were calculated based on the Cl/SO₄ ratio.

Pore water profile 13953 was chosen as an example of advective transport as indicated by the curvature of the Cl and Na profiles. Both start at sea water concentration and remained stable to about 50 cm sediment depth, after which they decreased to concentrations expected for groundwater in the area (Guglielmi & Mudry, 1996; Guglielmi & Prieur, 1997). pH increases to about 8 in the top 50 cm then declines again to about 7.4, followed by an increase to 8. The pH value of 8 at 110 cm should be evaluated as an outlier – potentially due to an unreliable measurement. Alkalinity shows a similar profile to pH with the exception of the outlier at 110 cm, however here variations are more pronounced. A sharp increase in the top 50 cm is followed by a sharp decrease over the next 40 cm. Ammonia increases to a depth of about 150 cm from seawater values to 80 mg/L and then decreases towards the bottom of off the profile to about 20 mg/L. Eh values are extremely negative in the top 50 cm where the seawater seems to be the dominating pore water source. This contrasts the Eh profile seen in core 13952, which was chosen to be the seawater example. Barium shows is very similar profile to that observed in core 13939 (see below), except that the highest barium value is already reached at about 120 cm sediment depth. The reactive layer in which Ba is released from the sediment to the pore water coincides with a sharp decline in sulfate. Thus the barite solubility is the likely control for its concentration in the pore water. Sulfate remains more or less stable in the top 50 cm of the rich there is a sharp decline over the next 50 cm down to a sediment depth of 100 cm, a depth at which its concentration is close to that expected for groundwater in southern France (Guglielmi & Mudry, 1996; Guglielmi & Prieur, 1997). This however is not likely explained by the presence of groundwater, but rather by microbial sulfate reduction. Based on the same assumption as for core 13952 a sulfate profile was calculated (Fig. 58). In the calculated profile groundwater concentrations are only reached at a depth of 210 cm. Interestingly is the decline in concentration much less dramatic than in the measured profile, indicating the presence of a pronounced reaction zone.

Pore water profile 13939, which is representative of diffusive transport (Fig. 59), shows more or less a linear decline in Cl and Na concentrations with depth. PH remains relatively stable at around 7.5 to 7.8 and only at the depths of 200 and 300 cm values approach those anticipated for seawater. Alkalinity and ammonia increase to a depth of about 200 to 220 cm after which they remain more or less constant. Barium values start at approximately seawater concentration and increase sharply to about 2 mg/L at a depth of 180 cm after

which the concentration decreases sharply but never falls below 0.6 mg/L. The sulfate concentration show the same linear decline, which was observed for Cl and Na, however, again sulfate values were slightly lower than expected (exclusively explained by mixing seawater and groundwater). The calculated profile shows much less variation and the decrease between 50 and 300 cm is more or less linear – exemplifying diffusion as the important process (Schlüter et al., 2004). Detailed results are also reported in Tables 3 and 4 (available on CD-ROM in back pocket).

Sediment geochemistry

The results for those cores, which were selected for chemical analyses by total digestions and ICP-OES are presented in Table 5 (available on CD-ROM in back pocket). Despite the large variation in pore water profiles and the presence of fresh water only little variation was observed in the chemical composition of the sediments. Most values varied within their analytical uncertainty. Particularly surprising was that Ba values did not vary, considering the large increases of Ba in the pore water fraction of core 13953 of up to 200-times seawater concentration. Despite the proximity to shore and, thus, the airport and industrial areas in the western part of Nice, the metals Cr, Cu, Ni, V and Zn were not elevated. Values corresponded to those expected for "normal" marine sediments (Li, 2000).

A second set of pore waters was recovered using rhizons (i.e. 50 ml syringes) stuck into the shallow subseafloor by scuba divers. The water samples were taken along the same transect along which *in situ* temperature measurements were carried out (see Ch. 7.3.3 above). Compared to the T loggers, pore water samples were extracted at transect positions 0, 10 (2), 20 (2), 25, 27, 30 (2), 35, 40 (2), 50, 60, 70, 80 (2), 90, 100, 110 and 120 m ([2] in parantheses means that a second sample was taken during a second dive). The corresponding water depths range between 15.5 mbsl at the NW' end of the transect and 29.5 mbsl at its SE' termination near the buoy (see Fig. 39, black bar). Pore water samples were taken along the transect every 10 m from a depth of approximately 5 - 10 cm. The purpose was to investigate if submarine groundwater discharge (SGD) was present in the area where most of the sediments, which were collected gravity coring showed a fresh component in their pore water. Results are presented in Table 6 (available on CD-ROM in back pocket) and none of the 13 samples indicates SGD – all measured concentrations were within their analytical uncertainty those expected for seawater.

Visual observations by SCUBA

In addition visual observations were made by SCUBA, which would indicate SGD, such as sediment discolorations, shimmering water, temperature anomalies and discrete vent orifices. No such features were observed. The only indication that there might be SGD was the discovery of minor microbial mats (Fig. 60), which may have developed due to a redox or nutrient gradient (e.g. Bussmann et al., 1999). Nevertheless, a pore water sample did not show any fresh water. As a consequence, we deduce that groundwater that may reach the deeper subseafloor successions owing to their increased permeability is not discharged into the ocean.

There is two obvious explanations for this observation. First, the hydraulic gradient may not be high enough to allow outflow into the overlying water body at a given depth. Second, the scuba divers may have missed the outcrops of the rather thin (usually <20 cm thick) beds of gravel which are charged with fresh water from the Var estuary and adjacent aquifer. Only long-term observations at multiple levels below the seafloor can shed unambiguous light on this open question.

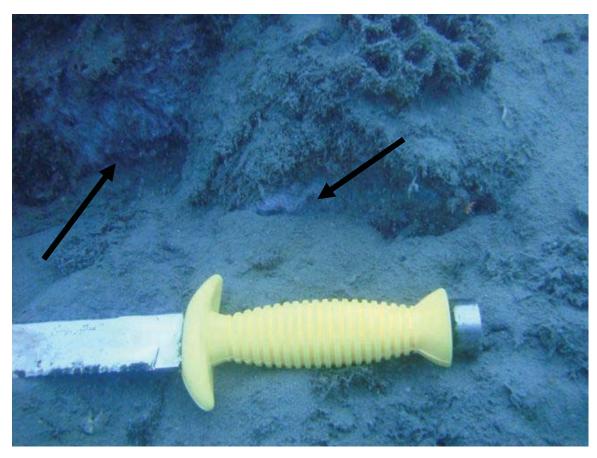


Fig. 60: Development of small microbial mats at the boundary between seawater and sediment (see arrows).

7.3.9. In situ Radon measurements

(A. Mayer, P. Henry)

Table 7 reports the results of the measurements. Figure 61 reports the vertical distribution of salinity in all radon measurement stations. It is evident from the figure that surface water contains a significant fraction of freshwater that, at the time of the measurement sessions, was almost certainly derived from the Var river, particularly during falling tides. In the limits of the salinity measurements precision, no significant input of freshwater from the seafloor is detected. Variations of salinity are interpreted as due to variable proportion of Var river water.

Radon concentrations are shown in Figure 62. The level of radon activity is quite low (just above the limit of detection of the method, i.e. ca. 1.5 Bq/m3) for most of the radon stations. Slightly higher activities are observed in zones of shallower water depth in the northern sector, but no localised radon input point is detected in this area, nor the gas flares.

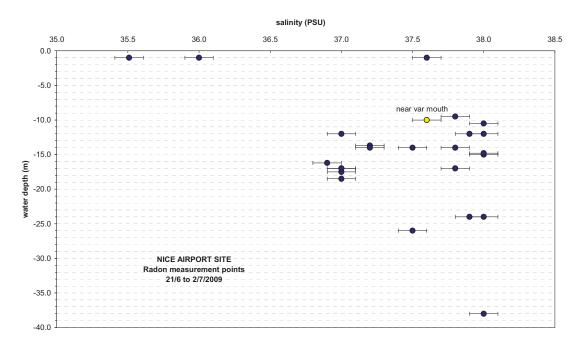


Fig. 61: Vertical distribution of salinity.

Figure 63 illustrates the vertical distribution of radon in seawater. Radon activity is low, but higher than the activity supported by ²²⁶Ra dissolved in seawater (blue vertical line) (Schmidt & Reiss, 1996). This is true in particular for the deepest station (-37 m), with salinity 38.0 psu, which yielded radon activity clearly higher than ²²⁶Ra supported activity. The slight excess of radon indicates that a slight contribution from pore or submarine ground water (GW) occurs in the water column. The figure also shows that for a given depth, north-

western measurements stations yielded slightly higher radon activities in respect to central and southern stations, suggesting that part of the radon 'excess' arises from eastward spreading of the Var river water (out of scale in the Fig. 63). Var river water, measured at the mouth, has a radon activity of 173 Bq/m3. Degassing and ageing of this water contribution reduce the radon net additions to seawater (arrows in Fig. 63).

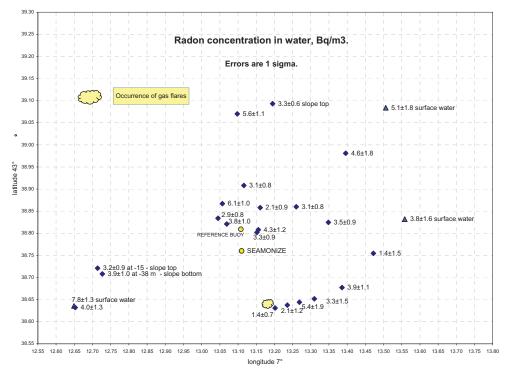


Fig. 62: Radon activity in the investigated area.

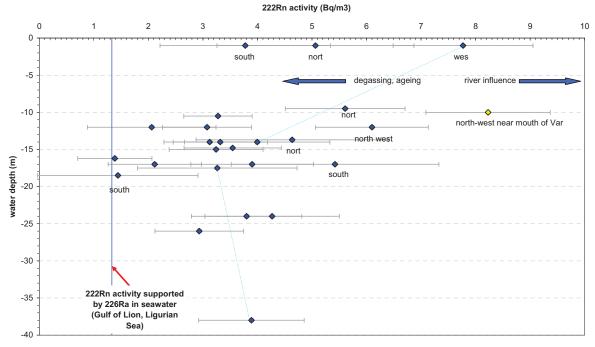


Fig. 63: vertical distribution of radon in seawater. North, south, west, indicates the location of the station sectors in respect to the main canyon (encroachment). Solid line indicates measurements made at different depth in the same stations.

Figure 64 illustrates the relationships between salinity and radon activity. About half of the measurement stations plots above the instantaneous mixing line between the sea water end-member and the Var river end-member. This line represents the maximum activity that can be achieved in seawater by mixing with the river water, since the mixing is considered instantaneous and degassing is neglected. Samples plotting above this line cannot be explained by simple radon addition and salinity sink due to mixing with Var river water, even considering the incertitude on the measurements. The results suggest that a small contribution of radon from groundwater of normal (or close to normal) salinity exists in the area. Alternatively, some radon may be input from re-suspended particles and diffusive exchange through the seafloor.

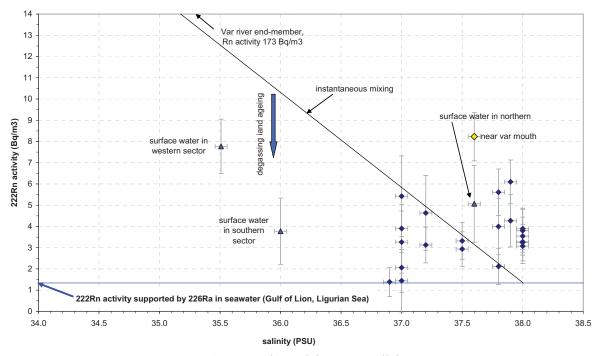


Fig. 64: Radon activity versus salinity.

Discharge rates

An estimate of the maximum discharge rate of the groundwater can be done using the measured radon activities and assumed radon activity of groundwater. Dissolved ²²⁶Ra in the Ligurian Sea accounts for about 1.33 Bq/m3 (Schmidt and Reiss, 1996). Taking an average water depth in the investigated area of 20m and an average radon activity of 4 Bq/m3, the excess inventory in the water column in respect to the dissolved ²²⁶Ra is 53 Bq/m2. For this inventory, the lost of radon by radioactive decay and atmospheric escape is estimated to be around 20 Bq/m2/day. To maintain in steady state the radon inventory, radon inputs from groundwater should occur. The necessary water flux could be calculated if radon activity in

groundwater would be known. Assuming an activity of groundwater of 3.5 kBq/m3, a common value measured in the Adriatic Sea and at the submarine spring of Port Miou near Marseille, the resulting water flux is ca. 5 litres / m2 seafloor / day, which correspond to a seepage rate of 5 mm /day. This result is of the same order as the discharge rate inferred by Guglielmi and Prieur (1997), although the latter neglected the contribution of Var river water and the contribution of recycled seawater. Large incertitude exists, of course, in this calculations due to unverifiable assumptions made above.

radon station	lat 43°	long 7°	Depth	Salinity	radon activity in water	
139xx-x	minutes	minutes	m		Bq/m3	abs error 1S
06-01	38.832	13.558	-1	36.0	3.8	1.6
06-01 and drifting to next	38.755	13.472	-18.5	37.0	1.4	1.5
06-02	38.677	13.386	-17	37.0	3.9	1.1
06-03	38.652	13.310	-17.5	37.0	3.3	1.5
06-04	38.644	13.269	-17	37.0	5.4	1.9
drifting between stations	38.638	13.235	-12	37.0	2.1	1.2
06-05	38.631	13.202	-16.2	36.9	1.4	0.7
09-01	39.084	13.506	-1	37.6	5.1	1.8
09-02	38.981	13.396	-13.7	37.2	4.6	1.8
16-01	38.860	13.260	-14	37.2	3.1	0.8
16-02	38.802	13.152	-14	37.5	3.3	0.9
22-1	38.858	13.161	-17	37.8	2.1	0.9
22-2 core42	38.834	13.045	-26	37.5	2.9	0.8
22-3	38.867	13.057	-12	37.9	6.1	1.0
22-4	38.821	13.069	-24	38.0	3.8	1.0
22-5	38.908	13.116	-12	38.0	3.1	0.8
43-1	39.070	13.098	-9.5	37.8	5.6	1.1
43-2	39.093	13.195	-10.5	38.0	3.3	0.6
44	38.636	12.649	-1	35.5	7.8	1.3
50-1	38.632	12.653	-14	37.8	4.0	1.3
50-2	38.721	12.714	-15	38.0	3.2	0.9
50-3	38.808	13.156	-24	37.9	4.3	1.2
50-4	38.825	13.348	-14.8	38.0	3.5	0.9
55	38.708	12.727	-38	38.0	3.9	1.0
61-1	39.256	12.006	-0.6	0.0	172.6	6.7
61-2	38.701	11.603	-10	37.6	8.2	1.1

Table 7: Results from radon measurements.

Perspective for the continuation of the research

In order to refine the calculation made above, radon activity should be measured in groundwater extracted from nearest piezometers. In addition, sediment water equilibration experiment should be performed to determine radon activity in pore water and radon diffusion rate from the sediments. A more detailed model of vertical diffusion in the water column and atmospheric escape should also be carried out to improve the estimate of the rate of radon inventory lost.

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10. Appendices

- 10.1 Station list
- 10.2 Lithologs and shear strength data
- 10.3 MSCL data logs and core photographs (electronic version only)

10.1 Station list

CALKS	LDR 20 18 13 20 18 24 03 20 18 24 03 20 18 33 04 20 19 07 02 20 19 07 02 20 19 12 38 20 19 12 38 20 19 12 38 20 10 05 17 20 20 05 12 20 20 05 12 20 20 05 12 20 20 05 12 20 20 44 40 20 20 44 40 20 20 44 40			ACTON @ Station	LAT 43.64845 43.64843	LON 7.22623 7.22623	DEPTH 18.6	WATER 24.2	WINCH	ENGH	EMARS
2009 00 00 00 00 00 00 00 00 00 00 00 00	18 18 24 18 24 18 33 19 00 11 19 00 12 10 12 10 12 10 12 10 12 10 10 12 10 10 10 10 10 10 10 10 10 10 10 10 10			@ Station	43.64845	7.22623	18.6	24.2			
2009 00 00 00 00 00 00 00 00 00 00 00 00	18 24 18 33 18 33 19 07 19 12 19 45 20 02 20 05 20 34 20 34 20 34 20 34 20 34			Ottobardon and and and and and and and and and an	43 64843	7 22608	707				
2009 00 00 00 00 00 00 00 00 00 00 00 00	18 24 19 33 19 07 19 08 19 45 20 02 20 05 20 34 20 34 20 34 20 34 20 34 20 34 20 34		Plezometer cast (CPT)	Station started	2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2. 2	00027.1	18.1	24.2			
2009 2009 2009 2009 2009 2009 2009 2009	18 33 19 01 19 07 19 08 19 45 20 02 20 05 20 05 20 05 20 05 44 20 44			to water	43.64843	7.22612	18.0	24.2			
2009 00 00 00 00 00 00 00 00 00 00 00 00	19 01 19 07 19 08 19 45 20 02 20 05 20 05 20 34 20 34 20 34 20 34 44	$\Box\Box$		at bottom	43.64842	7.22610	18.1	24.2	W2	25	Piezometer remains 30 min in bottom
2009 00 00 00 00 00 00 00 00 00 00 00 00	19 07 19 08 19 12 20 02 20 05 20 34 20 34 20 34 20 34 44	\neg	Piezometer cast (CPT)	heave started	43.64840	7.22587	17.7	23.9	W2	25	
2009 06 2009 06 2009 06 2009 06 2009 06 2009 06 2009 06 2009 06 2009 06 2009 06	19 08 19 12 20 02 20 05 20 05 20 34 20 34 44 47		Piezometer cast (CPT)	at surface	43.64818	7.22567	17.1	24.0	W2	25	
2009 06 2009 06 2009 06 2009 06 2009 06 2009 06 2009 06 2009 06 2009 06	19 12 20 02 20 05 20 05 20 34 20 36 20 44 47	7-10		to water	43.64812	7.22572	17.7	23.9	W2	25	
2009 06 2009 06 2009 06 2009 06 2009 06 2009 06 2009 06	20 02 20 05 20 05 20 05 20 05 20 05 20 05 20 34 44 47 47 47 47 47 47 47 47 47 47 47 47		Piezometer cast (CPT)	at bottom		7.22568	17.1	23.9	W2	25	
2009 06 2009 06 2009 06 2009 06 2009 06 2009 06	20 02 20 05 20 34 20 36 20 36 44 44 47			at surface	43.64805	7.22530	16.1		W2	25	
2009 06 2009 06 2009 06 2009 06 2009 06 2009 06	20 05 20 34 20 36 20 44 20 47	01-3	Piezometer cast (CPT)	to water		7.22467	16.4	23.9	W2		
2009 06 2009 06 2009 06 2009 06 2009 06	20 34 20 36 20 44 20 47		Piezometer cast (CPT)	at bottom	43.64725	7.22455	16.1	24.0	W2	21	
2009 06 2009 06 2009 06 2009 06	20 36 20 44 20 47	_	Piezometer cast (CPT)	heave started	43.64727	7.22457	15.9	24.1	W2	22	heave with 0,3 m/s
2009 06 2009 06 2009 06	44			at surface	43.64723	7.22458	16.7				
2009 06	47	410		to water	43.64662	7.22375	14.8	24.1	W2		
2009 06			Piezometer cast (CPT)	at bottom	43.64655	7.22368	14.6	24.2	W2	20	
	20 21 16 09			heave started	43.64658	7.22372	14.7	24.2		20	
П	20 21 18 47			at surface	43.64642	7.22377	14.8	24.2	W2	20	
2009 06	20 21 30 39	01-5	Piezometer cast (CPT)	to water	43.64585	7.22278	14.5	24.2	W2	20	
POS-386 2009 06	20 21 32 20			at bottom	43.64585	7.22272	14.7	24.2	W2	19	
	20 21 59 19			heave started	43.64585	7.22255	14.1	24.3	W2	19	
2009 06	20 21 59 42		Piezometer cast (CPT)	heave started	43.64585	7.22253	14.0	24.3	W2	19	
POS-386 2009 06	20 22 02 38	-	Piezometer cast (CPT)	at surface	43.64565	7.22283	14.9	24.2	W2	19	
2009 06	20 22 07 59			on deck	43.64567	7.22305	15.2	24.1	W2	19	
2009	╗	╗	CPT)	Station completed	43.64568	7.22312	15.3	24.1	W2	19	
2009	7	02-1		Station arrived	43.57508	7.23822	1279.0	22.8	W3		
POS-386 2009 06	1		Mooring ANIIT RA	anchor to water	43.57500	7.23818	1279.0	22.8	W3		
5000	n 0			Allilla to water	45.0700	7.0000	12/ 9.0	6.23	200		
5000	00		Mooring ANIII KA	Anlitra dived	43.57510	7.23828	1280.0	6.22		707	11.00
	34		Mooring ANIII KA	Mooring at bottom	43.57508	7.23825	1280.0	92.59		1045	Hydrophone to water
5002	35		Mooring ANIII KA	released, heave started	43.57507	7.23825	1280.0	22.9		1045	Hydrophone on deck
5002	84		Mooring ANIII KA	upper releaser at surface		7.23858	1280.0		W3		1,77
2009 06	08 49	П	Mooring ANIII RA	upper releaser on deck		7.23865	1279.0		W3		station completed
2006 000	23	7-20	Ibsen Mooring	to water		7.23853	12/9.0		Z/M	4070	station started
	67 6			at bought	45.57557	7 22030	1270.0	22.9	WZ	1270	Hydrophone to water
2000	30			released, fleave staffed	45.37337	7 22040	12010	22.9	VVZ VVO	17/0	Trydiculate of deck
FUS-300 2009 00 100 00 00 00 00 00 00 00 00 00 00 00	21 09 45 20	00 00	Mooring Door Cont	releaser of deck	43.37300	7 22782	1201.0	0.22	ZW		station completed
2000	24 11 10 34	T		stalt stackling out	43.37313	7 23870	1285.0	0.22	2/\		Station stated
2009	T			station completed	43.57398	7 24140	1296.0	22.0 22.8	WZ		
	21 11 52 43	3 03-1		Head Blov to water	43.62472	7 18250	508.0	23.7	744		Station arrived
2009	Π	Π		anchor weight to water	43.62360	7.18345	515.0	23.6			
POS-386 2009 06	Г			Head buoy dived	43.62332	7.18343	518.0	23.6			station completed
		03-2	Ibsen Mooring	station arrived	43.62465	7.18410	500.0		W2		
POS-386 2009 06	21 12 14 37		Ibsen Mooring	to water	43.62467	7.18417	501.0	23.7	W2		Benthos + Releaser t/water
86 2009 06	21 12 34 51		Ibsen Mooring	atbottom	43.62450	7.18352	505.0	23.6	W2	490	Hydrophone t/water - Benthos released
	21 12 35 10			heave started	43.62450	7.18350	506.0	23.6	W2	490	Hydrophone t/water - Benthos released
386 2009 06	21 12 35 36			heave started	43.62448	7.18350	508.0	23.6		490	Hydrophone @ deck
POS-386 2009 06	21 12 42 12			on deck	43.62472	7.18335	504.0	23.8	W2		Releaser on deck
POS-386 2009 06	43			Station completed	43.62485	7.18340	503.0	23.8	W2		
2009 06	33	7 04-1		Station arrived	43.64673	7.18633	131.0	23.2			
2009 06	21 13 34		Ibsen / Peer Gynt (VH) Mooring	to water		7.18630	137.0	23.2			Top buoy t/water
POS-386 2009 06	21 13 35 35		Ibsen / Peer Gynt (VH) Mooring	to water	43.64660	7.18617	128.0	23.3			Trap + ADCP t/water

2009 06 21 13 37 54	Ibsen / Peer Gynt (VH) Mooring	to water	43.64637	7.18602	117.0			Deadweight t/water
06 21 13 42	Ibsen / Peer Gynt (VH) Mooring	to water	43.64618	7.18597	128.0			Benthos + Releaser
2009 06 21 13 51	Ibsen / Peer Gynt (VH) Mooring	at bottom	43.64617	7.18577	124.0			Hydrophone t/water + Benthos released
2009 06 21 13 51	Ibsen / Peer Gynt (VH) Mooring	heave started	43.64613	7.18577	124.0		W2 108	Hydrophone @ deck
06 21 13 53 47	Ibsen / Peer Gynt (VH) Mooring	Station completed	43.64617	7.18582	126.0		W2	Releaser @ deck
	Piezometer cast (CPT)	Station arrived	43.64710	7.22582	19.1	23.4 V	W2	
POS-386 2009 06 21 14 53 47	Piezometer cast (CPT)	to water	43.64718	7.22600	19.8	23.3 V	W2	
POS-386 2009 06 21 15 05 40	Piezometer cast (CPT)	slack started	43.64713	7.22613	20.1	23.3 V	W2	
	Piezometer cast (CPT)	at bottom	43.64707	7.22608	19.0			WD = 20m
POS-386 2009 06 21 15 35 32	Piezometer cast (CPT)	heave started	43.64695	7.22588	19.0	23.2 V	W2 26	WD = 20m
POS-386 2009 06 21 15 40 51	Piezometer cast (CPT)	on deck	43.64740	7.22630	20.3	23.2 V	W2 26	WD = 20m - WL max = 43m
POS-386 2009 06 21 16 00 08 05-2	Piezometer cast (CPT)	Station arrived + to water	43.64653	7.22483	17.3		V2	
POS-386 2009 06 21 16 04 16	Piezometer cast (CPT)	slack started	43.64652	7.22482	17.2	23.2 V	W2	
POS-386 2009 06 21 16 05 07	Piezometer cast (CPT)	at bottom	43.64653	7.22482	17.1	23.2 V	W2 24	Boko = 19m, WL max =
POS-386 2009 06 21 16 34 04	Piezometer cast (CPT)	heave started	43.64658	7.22480	17.2	23.4 V		Boko = 19m, WL max = 33
POS-386 2009 06 21 16 37 38	Piezometer cast (CPT)	atsurface	43.64650	7.22488	17.5	23.4 V	W2 24	
POS-386 2009 06 21 16 42 10 05-3	Piezometer cast (CPT)	Station completed	43.64617	7.22508	19.5	23.3	W2 24	
POS-386 2009 06 21 16 50 58	Piezometer cast (CPT)	Station arrived + to water	43.64535	7.22420	20.5	23.2 V	WZ	
POS-386 2009 06 21 16 54 50	Piezometer cast (CPT)	at bottom	43.64532	7.22420	20.7		W2 27	BoKo= 23 m
POS-386 2009 06 21 17 27 02	Piezometer cast (CPT)	heave started	43.64533	7.22420	20.9	23.3 V	V2 27	WL= 42 m
POS-386 2009 06 21 17 28 49	Piezometer cast (CPT)	on deck	43.64527	7.22427	21.0		WZ	
POS-386 2009 06 21 17 31 18	Piezometer cast (CPT)	Station completed	43.64540	7.22427	18.8	23.4 V	W2	
POS-386 2009 06 21 17 49 48	Piezometer cast (CPT)	Station arrived + to water	43.64482	7.22303	19.0	23.2 V	WZ	
POS-386 2009 06 21 17 59 07	Piezometer cast (CPT)	at bottom	43.64468	7.22318	19.2	23.3 V	W2 27	BoKo=27m
POS-386 2009 06 21 18 28 54 05-4	Piezometer cast (CPT)	heave started	43.64480	7.22307	18.4	23.4 V	W2	
POS-386 2009 06 21 18 31 58	Piezometer cast (CPT)	at surface	43.64458	7.22295	19.6	23.4 V	W2	station completed
POS-386 2009 06 21 18 49 33 05-5	Piezometer cast (CPT)	to water	43.64418	7.22177	18.0	23.5 V	W2	station started
POS-386 2009 06 21 18 53 26	Piezometer cast (CPT)	at bottom	43.64423	7.22177	17.8	23.4 V	W2 26	Boko 26m
POS-386 2009 06 21 19 23 17	Piezometer cast (CPT)	heave started	43.64425	7.22185	17.3	23.4 V	W2 29	
POS-386 2009 06 21 19 24 27	Piezometer cast (CPT)	at surface	43.64425	7.22187	17.1		W2	station completed
POS-386 2009 06 21 19 47 05 05-6	Piezometer cast (CPT)	to water	43.64407	7.22115	17.0	23.2 V	W2	station started
POS-386 2009 06 21 19 50 02	Piezometer cast (CPT)	at bottom	43.64405	7.22115	16.8	23.2	W2 27	Boko 27 m
POS-386 2009 06 21 20 19 42	Piezometer cast (CPT)	heave started	43.64410	7.22122	16.7		W2	
2009 06 21 20 20 36	Piezometer cast (CPT)	on deck	43.64412	7.22125	16.5	0	W2	
POS-386 2009 06 21 20 40 31 05-7	Piezometer cast (CPT)	to water	43.64385	7.22003	15.9	23.1 V	W2	
06 21 20 42	Piezometer cast (CPT)	slack started	43.64385	7.22017	16.0			
2009	Piezometer cast (CPT)	at bottom	43.64387	7.22017	15.9		W2 25	
2009 06 21 21 14	Piezometer cast (CPT)	heave started	43.64397	7.22018	15.6		W2 25	
06 21 21 14	Plezometer cast (CPT)	on deck	43.64388	7.22023	15.7		WZ	
2009 06 21 21 45 19	Plezometer cast (CPT)	Station completed	43.64378	01.022.7	6.61	Z3.0 V	WZ	
00000	MI SIGNOIS HOLL GECK	7	40 77000	7 50400	0447		9	
POS-300 2009 00 22 00 43 34 0/-1	Mooring MAP 2	Station arrived	43.55003	7 50275	2474.0	V 20.4	6/1/2	
2009 00 27 00 40	Mooring MAD 2	to water	49.55000	7 50075	0.471.0	Ī	CAA	Manipoli cot oi maino all
2009 00 22 00 30	Macaille MARY 2	ni deck	43.33020	7.30373	0.4712		2 5	MOUTING IS TOO IIGHT ::::
06 22 07 40	Mooring MAP 2	to water	43.55005	7 58322	2174.0	V 6.02	W3	
2009 00 22	Mooring MAD 2	at deptil	42 55000	7.50007	0.471.0			
2009 00 22 09 12	Mosting MAP 2	riyaropirone to water	43.33000	7 50245	0.4712			
22 00	Mooring MAD 2	house of the deliberation	49.55010	7 500.10	2400.0	0.02	2000	
2009 06 22 09 13	MOOTING WAY 2	neave started	43.330112	7.003.7	2138.0			
2009	Mooring MAP 2	releaser on deck	43.54917	7 50247	0.6712		W3	
17 CS 60 77 Q0	Mooring MAP 2	Station completed	43.34913	7.75743	0.6712		W3	
2009 06 22 10 53	Borel Mooring	Station arrived	43.52167	7.55710	2136.0	20.8		
2009 06 22 10 54	Borel Mooring	Head buoy to water	43.52165	7.55708	7130.0	8.0.8		
POS-360 2009 00 22 11 02 36	Borel Mooring	on deck	43.52077	7.557.52	1708.0	20.02		C trial
2003 00 22 11 33		neau buoy to water	49.30117	1.01012	0.0671	20.02		ZIN UIAI

				I	ſ	ſ			
5003	T		start slacking line				20.8		
2009 06 22 12	П		1st 600 m slacked out			2012.0	21.1		
	50 08-1)	to water	43.65147		57.0	23.0		station arrived
	42								Boko 77 m
POS-386 2009 06 22 17 32	26)	heave started	43.65143	7.22513		23.0 W2	83	
	59	Piezometer (CPT)					23.0 W2		Station completed
POS-386 2009 06 22 17 53	33 08-2	Piezometer (CPT)			7.22322	13.1	23.0 W2		Station started
POS-386 2009 06 22 17 56	29			43.64965	7.22327		23.0 W2	2 24	Boko 24 m
2009	П		ted				23.1 W2		
	32	Piezometer (CPT)	on deck			13.1	23.1 W2		
POS-386 2009 06 22 18 57	26		on deck	43.64155	7.22738	48.2	23.0 W2		Station work ceased
П	60	om deck							
POS-386 2009 06 23 06 07	18		Station started			1552.0	20.5 W3	~	
POS-386 2009 06 23 06 08	20		to water	43.52903		1533.0	20.5 W3		
	27			43.52925					Boko 1722 m
POS-386 2009 06 23 06 41	42		pe		7.35602	1529.0	20.7 W3	1730	
POS-386 2009 06 23 07 09	18	Gravity Corer	at surface	43.52925		1540.0	20.7 W3	3	
POS-386 2009 06 23 07 10	32			43.52933			20.7 W3	~	station completed
POS-386 2009 06 23 07 55	20 11	Gravity Corer	pei	43.53502	7.35182	1417.0	21.0 W3	~	
POS-386 2009 06 23 07 55	31	Gravity Corer		43.53503	7.35180	1417.0	21.0 W3	~	
POS-386 2009 06 23 08 22	18	Gravity Corer					21.0 W3	1590	Boko 1580m
POS-386 2009 06 23 08 22	41	Gravity Corer		43.53500	7.35167	1418.0	21.0 W3	1590	Boko 1580m
POS-386 2009 06 23 08 48	58	Gravity Corer			7.35205	1424.0	21.1 W3	3	
POS-386 2009 06 23 08 49	41					1423.0	21.1 W3		
POS-386 2009 06 23 10 16	15 12			43.64635	7.21858		23.1 W2		
POS-386 2009 06 23 10 32	00			43.64638	Г	29.3	23.0 W2		
POS-386 2009 06 23 10 34	40			43.64620					
POS-386 2009 06 23 10 36	46					26.0			
POS-386 2009 06 23 11 19	14			43.64648	7.21842	33.7	22.8 W5a	ža	
POS-386 2009 06 23 12 28	53		Station suspended			159.0	23.4 W5a	'a	ROV scrap
POS-386 2009 06 23 14 55	02	MAP 2			7.57897		21.0		Hydrophone to water
POS-386 2009 06 23 14 56	02	MAP 2		43.54577	7.57922		21.0		
POS-386 2009 06 23 15 50	46			43.55032	7.58140		21.0		hydrophone on deck
POS-386 2009 06 23 16 00	0.1			43.54705	7.57570		21.0		
POS-386 2009 06 23 16 05	01	MAP 2		43.54740	7.57683		21.0		
POS-386 2009 06 23 17 00	33		_		7.57972				
T	25 13		rived	43.51085	7.35648	П			
2009	25								
POS-306 2009 06 24 04 30	- 12	Gravity corel	at bottom	43.01000	7.35640	1769.0	Z1.1 W3	1907	DURU 1097 III
Ì	780				Ī	1763.0	21.1 W3	T	station completed
	03 14			43.52252		1630.0	21.2 W3		
POS-386 2009 06 24 05 49	02		to water	43.52247	Γ	Γ	21.2 W3		
POS-386 2009 06 24 06 18	11			43.52250	7.35333	1650.0	21.1 W3	1794	Boko 1784m
POS-386 2009 06 24 06 18	19				7.35332	1650.0	21.1 W3		Boko 1784m
POS-386 2009 06 24 06 48	00	ity corer	on deck	43.52262	7.35338	1647.0	21.1 W3	_	Boko 1784m
POS-386 2009 06 24 08 11	31 15-1		Station started	43.64778		13.3	22.4 W2		
POS-386 2009 06 24 08 12	42	CPT	to water	43.64778	7.22097		22.4 W2		
	43								
	47		pe						
POS-386 2009 06 24 09 22	10		at surface	43.64775	7.22087	14.1	22.8 W2	2 20	
	23	CPT							
T	49		tarted		T				
06 24 09	Т	CP.	to water	43.64672	7.21918		22.9 WZ		Otalian constant
PUS-300 ZUUS U0 Z4 US 43	70					6.02	27.3		Station ceased

٦	1					40,400,6	1 000	0 0000	0 00		_	
	06 24	15 53	08 17	Gravity Corer	station arrived	43.40005	0.00055	2.761.0	50.9	W3		
POS-386 2	90	15 56	60	Gravity Corer	to water	43.39993	7.66673	2260.0	21.1	M3		
	24	T	24	Gravity Corer	at depth	43.39968	7.66658	2260.0	21.1	W3	2180	
	00 24	10 30	20	Gravity Corer	at depth	43.39970	1,000.1	0.1.022	1.12	W3	2431	BOKO 2421M
Т	90	12 00	00 00		neave staned	43.39972	1/000.1	2260.0	71.1	W3	743	BOKO 242 IIII
200-200	2009 00 24	17 10	60	Gravity Core	at sunace	43.39962	1100011	2261.0	- 17	6///		
T	3 8	2 0	Τ	Glavity Colei	station completed	40.03300	7.00220	2420.0	0.12	200		T-4411
T	00	00 01	02 07-3	Borel Buoy	station started	43.54330	7/505/7	21/5.0	67.7			Changing the Transceiver
POS-386 2	90	10 11	20		picked up	43.54248	7.56662	2174.0	23.1			Changing the Transceiver
T	90	10	54		heave started	43.54250	7.56660	2174.0	23.1			Changing the Transceiver
	2009 06 26	10 14	22	Borel Buoy	on deck	43.54243	7.56660	2174.0	23.0			fixed at portside of the ship & prepared for working
POS-386 2	2009 06 26	10 15	33	Borel Buoy	on deck	43.54237	7.56668	2177.0	23.0			Work for changing the Transceiver starts
	2009 06 26	10 51	17	Borel Buoy	on deck	43.54243	7.56650	2174.0	23.0			Work for changing the Transceiver finished
Г	2009 06 26	10 51	46		slack started	43.54242	7.56648	2174.0	22.9			
	90	10 52	36		to water	43.54240	7.56647	2174.0	22.9			
POS-386	2009 06 26	10 53	60		Station completed	43.54238	7.56648	2174.0	22.9			
Г	90	12 52	08		Station started	43.54032	7.56855	2173.0	23.3			
Ī	90	12 53	28		picked up	43.54025	7.56897	2172.0	23.3			
POS-386 2		12 54	39		heave started	43.54023	7.56903	2173.0	23.3			
	2009 06 26	12 56	23		on deck	43.54020	7.56905	2172.0	23.2			fixed on portside & start with work
Ī	06 26	14 06	18		slack started	43.54008	7.56920	2172.0	23.2			work is finished
Γ	2009 06 26	14 06	30		to water	43.54008	7.56918	2172.0	23.2			
Т	06 26	14 07	12		Station completed	43 54032	7 56895	2172 0	23.2			
T	06 27	08 27	16 18	, d	Station arrived to water	43 62787	7 21362	396.0	22.5	W3		
POS-386	2009 06 27	08	Т		atbottom	43 62770	7.21407	397.0	22.1	W3	437	Boko 429
T	2009 06 27	08 33	22		heave started	43.62770	7.21405	397.0	22.1	W3	437	Boko 429
	06 27	08 43	41		on deck	43.62782	7.21388	399.0	22.2	W3		
Ī	2009 06 27	08 46	13		station completed	43.62788	7.21402	394.0	22.2	W3		
	2009 06 27	00 60	21 19	Gravity Corer	station arrived	43.64297	7.22252	37.5	23.0	W3		
	06 27	09 13	00		to water	43.64322	7.22260	45.2	22.8	W3		
	2009 06 27	12	22	Gravity Corer	at bottom	43.64317	7.22248	40.8	22.9	W3	44	Boko 35m
	27	09 18	30		at surface	43.64322	7.22255	39.9	22.9	W3		
	06 27	09 19	02		on deck	43.64322	7.22253	38.3	22.9	W3		station completed
П	2009 06 27	09 33	24 20	Gravity Corer	station arrived	43.64528	7.22375	18.9	23.1	W3		
POS-386 2	2009 06 27	09 48	10		to water	43.64527	7.22382	19.4	22.8	W3		
	2009 06 27	09 60	28	Gravity Corer	at bottom	43.64527	7.22373	18.9	22.6	W3	30	Boko 22
	2009 06 27	09 60	45	Gravity Corer	heave started	43.64527	7.22367	18.9	22.6	W3	30	
POS-386	2009 06 27	09 53	25		on deck	43.64532	7.22367	18.6	22.7	W3	30	
	2009 06 27	60	43		Station completed	43.64533	7.22365	18.1	22.8	W3	30	
	2009 06 27	11 07	02 21-1		station arrived	43.64417	7.21327	0.09	22.9	W2		
POS-386 2	2009 06 27	11 08	13	CPT pilum	to water	43.64415	7.21313	59.0	22.8	W2		
	2009 06 27	11 19	22		at bottom	43.64422	7.21338	61.0	23.0	W2	62	
POS-386 2	2009 06 27	11 51	60	CPT pilum	heave started	43.64417	7.21332	61.0	23.0	W2	92	
	2009 06 27	11 52	40	CPT pilum	at surface	43.64417	7.21332	61.0	23.0	W2		
	2009 06 27	12 10	02 21-2	CPT pilum	to water	43.64452	7.21422	61.0	23.1	W2		
П	2009 06 27	12 18	27		at bottom	43.64447	7.21415	62.0	23.3	W2	80	
П	2009 06 27	12 29	03		heave started	43.64448	7.21423	62.0	23.2	W2	80	
	2009 06 27	12 33	20	CPT pilum	at surface	43.64447	7.21412	62.0	23.1	W2	5	
	2009 06 27	12 42	02 21-3	CPT pilum	to water	43.64452	7.21433	62.0	23.1	W2		
	2009 06 27	12 47	11	CPT pilum	at bottom	43.64452	7.21438	62.0	23.4	W2	75	Boko 69m
П	2009 06 27	13 20	43		heave started	43.64453	7.21410	61.0	23.2	W2	75	Boko 69m
POS-386 2	2009 06 27	13 21	54	CPT pilum	at surface	43.64510	7.21557	20.0	23.4	W2		
	27	13 36	34 21-4	milia LOO	to motor	0727007	1000	0 01	, ,,,	9::		
					IO Water	43.64510	7.21557	50.0	23.4	WZ		

2009 06 27 14 19	CPT pilum	ted	43.64513	7.21527	55.0	23.4	72	Boko 52m
POS-386 2009 06 27 14 22 09	CPT pilum	at surface	43.64512	7.21552	51.0	23.3 W2		
	CPT pilum	on deck	43.64500	7.21597	62.0	23.3 W2		
POS-386 2009 06 27 14 54 00 21-5		ved	43.64268	7.21010	15.4	23.6 W2		
	CPT pilum		43.64272	7.21000	15.2	23.5 W2	2	
POS-386 2009 06 27 15 01 31	CPT pilum	at bottom	43.64265	7.21022	15.5	23.5 W2	19	Boko 16m
POS-386 2009 06 27 15 31 20		per	43.64283	7.21037	15.1	23.4 W2	38	
POS-386 2009 06 27 15 34 58		at surface	43.64297	7.20988	15.3	23.3 W2		
2009 06 27 15 51			43.64275	7.20998	15.4			station completed
POS-386 2009 06 27 16 04 02 21-6	CPT pilum		43.64310	7.21097	14.0	23.4 W2		station arrived
POS-386 2009 06 27 16 07 03	CPT pilum	at bottom	43.64303	7.21110	14.1	23.5 W2	19	Boko 16m
			43.64302	7.21105	14.9	23.3 W2	26	Boko 16m
POS-386 2009 06 27 16 39 36	CPT pilum		43.64303	7.21100	14.6			
POS-386 2009 06 27 16 59 10 21-7		to water	43.64375	7.21258	25.4	23.5 W2	2	
П			43.64377	7.21247	21.6		22	Boko 17m
POS-386 2009 06 27 17 31 40		heave started	43.64363	7.21265	36.7	23.5 W2	52	Boko 17m
POS-386 2009 06 27 17 35 23		at surface	43.64373	7.21278	34.5	23.4 W2		
POS-386 2009 06 27 17 39 56	CPT pilum	on deck	43.64380	7.21260	30.8	23.4 W2		station completed
POS-386 2009 06 27 18 51 00 23-1	Bobcore	rived	43.64285	7.21043	15.1	23.2		
POS-386 2009 06 27 18 51 35			43.64288	7.21040	15.0			
POS-386 2009 06 27 18 53 28			43.64287	7.21027	17.4	23.2 W2	20	Boko 15
POS-386 2009 06 27 18 54 39		at surface	43.64283	7.21027	18.8	23.3 W2	20	Boko 15
POS-386 2009 06 27 18 54 49	Bobcore		43.64285	7.21028	16.6		20	Boko 15
POS-386 2009 06 27 18 56 27		pleted	43.64290	7.21035	15.6	23.2 W2	20	Boko 15
POS-386 2009 06 27 19 04 12 23-2		station arrived	43.64323	7.21092	14.4	22.8 W2		
POS-386 2009 06 27 19 11 07	Bobcore	to water	43.64312	7.21083	14.4			
POS-386 2009 06 27 19 13 44	Bobcore		43.64322	7.21075	14.4		22	Boko 16m
POS-386 2009 06 27 19 13 59			43.64320	7.21077	14.3	22.8 W2	22	Boko 16m
2009 06 27 19 16			43.64333	7.21090	14.4		22	Boko 16m
06 27 19 18 14			43.64325	7.21058	14.4			
2009 06 28 06 14	anchor deployment for scuba divers		43.64668	7.21867	25.3			
06 28 06 15	anchor deployment for scuba divers	eight to water	43.64668	7.21865	25.4			
2009 06 28 06 18	anchor deployment for scuba divers		43.64675	7.21867	23.4			interrupted, anchorline to short
2009 06 28 06 24	anchor deployment for scuba divers	to water	43.64695	7.21862	16.6			
06 28 06 27 36	anchor deployment for scuba divers	ted	43.64608	7.21807	34.2			
2009	29	rrived	43.64603	7.21863	29.3			
POS-386 2009 06 28 06 41 02	သ္ဌာ		43.64600	7.21860	29.4		42	20
06 28 06 50	25 05	on deck	43.64607	7 21855	30.6	22.3 W3	74	Banana
2009 06 28 07 31	25	npleted	43.63362	7.20772	358.0			Banana
POS-386 2009 06 28 07 55 05 26			43.64482	7.22222	16.2			
POS-386 2009 06 28 08 16 37		to water	43.64507	7.22380	20.5			
POS-386 2009 06 28 08 18 48			43.64507	7.22368	19.8		25	Boko 17m
POS-386 2009 06 28 08 18 53	GC		43.64507	7.22367	19.8	22.5 W3	25	Boko 17m
2009 06 28 08 24			43.64508	7.22380	20.6			
2009 06 28 08 24 57	35	pe	43.64508	7.22387	20.8			
06 28 08 53	CPT pilum	station arrived	43.64667	7.21725	32.5	22.4 W2		
2009 06 28 09 03			43.64668	7.21737	34.4			
2009 06 28 09 16			43.64675	7.21728	34.4		41	Boko 36
06 28 09		on deck	43.64658	7.21727	32.7	22.4 W2		
2009 06 28 09 53 35	CPT pilum		43.64657	7.21730	34.1			
2009 06 28 10 02		IVed	43.64612	7.27735	47.6			
POS-386 2009 06 28 10 02 32 POS-386 2009 06 28 10 17 07	Enlig Lac	to water	43.64615	7 24757	42.2	22.3 WZ	52	Boko 48m
71 01 02 00 5007			10.04000	10112.1	2.4		76	DONO 4011

POS-386 2009 06 2	8 10 47	7 41	CPT pilum	heave started	43.64590	7.21750	47.1	22.5		Boko 48m
2009 06	28 10 49	3 20	CPT pilum	at surface	43.64597	7.21752	45.9		W2 58	
	28 10 59	9 49 27-3	CPT pilum	to water	43.64608	7.21838	30.2	22.6	W2 5	
POS-386 2009 06 2	38 11 03	3 58		at bottom	43.64608	7.21827	32.1	22.6	W2 39	Boko 35m
90	28 11 34	1 26		heave started	43.64598	7.21837	30.7		W2 49	Boko 35m
2009 06	28 11 36	3 24		at surface	43.64605	7.21828	31.7	72.7	N2	
POS-386 2009 06 2	12 01	33 27-4		to water	43.64627	7.21915	20.2	22.7	W2	
POS-386 2009 06 2	28 12 06	3 07	CPT pilum	at bottom	43.64627	7.21920	18.5	722.7	W2 31	BoKo 26
2009 06	28 12 34	1 28		heave started	43.64630	7.21933	18.6		N2 32	
2009 06	28 12 35	5 47		at surface	43.64628	7.21945	17.8	22.6	W2	
2009 06	28 12 36	3 18	CPT pilum	on deck	43.64628	7.21943	17.8	22.6	W2	
2009 06	28 12 41	38		station ceased	43.64620	7.21968	17.6		V2	
2009 06	28 13 37	17		station cont.	43.64665	7.22015	16.7		W2	
POS-386 2009 06 2	28 13 37	7 26 27-5		to water	43.64665	7.22015	16.8	22.8	N2 5	
2009 06	28 13 58	3 54	CPT pilum	at bottom	43.64662	7.22005	16.6		W2 23	Boko 16m
2009 06	28 14 33	38		heave started	43.64657	7.22017	16.7			Boko 16m
2009 06	28 14 36	3 58	CPT pilum	at surface	43.64672	7.22000	16.7	23.3	W2	
2009 06	28 14 38	3 20	CPT pilum	station completed	43.64680	7.21987	17.0	23.3	N2	CPT broken
POS-386		22-2	Rn stations from Poseidon 3							
POS-386 2009 06 2	29 05 58	32 28	GC	station arrived	43.64133	7.21372	100.0	22.1	W3	
2009 06	29 06 04	111		to water	43.64132	7.21327	82.0			
POS-386 2009 06 2	90 90 6	3 42		atbottom	43.64133	7.21332	79.0	22.2	W3 84	Boko 76 m
POS-386 2009 06 2	90 90 67	3 55		heave started	43.64133	7.21332	83.0	22.2	W3 84	
2009 06	29 06 10	18	29	on deck	43.64128	7.21318	0.06		W3	
POS-386 2009 06 2	90 60	54	29	station completed	43.64128	7.21318	0.06	22.2	W3	
POS-386 2009 06 2	59 06 57	7 09 29	29	station arrived	43.64597	7.21402	29.8		W3	
2009 06	29 06 58	3 51		to water	43.64600	7.21385	29.0		W3	
POS-386 2009 06 2	29 07 00	18		at bottom	43.64607	7.21383	28.8	22.3	W3 34	Boko 26m
2009 06	29 07 00	39		heave started	43.64607	7.21385	28.9	22.3	W3	Boko 26m
2009 06	29 07 03	3 18	29	on deck	43.64610	7.21387	29.3		N3	
2009 06	29 07 03	38		station completed	43.64610	7.21388	59.9	22.3	W3	
2009 06	29 07 31	08 30		station arrived	43.64603	7.21825	40.8	22.4	W3	
2009 06	29 07 32	36		to water	43.64605	7.21825	40.4		W3	
2009 06	29 07 34	1 25		at bottom	43.64610	7.21822	41.0	22.4	W3 42	Boko 34m
2009 06	29 07 39	٤ 00	95	on deck	43.64608	7.21822	41.1	22.3	W3	Banana
90	29 10 52	2 56 31		station arrived	43.64565	7.21782	40.3		W3	
2009 06	29 11 33	3 48		to water	43.64603	7.21798	35.3		W3	
2009 06	29 11 36	20		atsurface	43.64605	7.21830	31.5			
90	29 11 37	27		at depth	43.64605	7.21825	31.4			
POS-386 2009 06 2	11 47	7,00	Seamonice	at surface	43.64593	7 24 02 0	30.2		W3 20	
2009 000	1 10	25 62		ar de l'inche	42.64600	7.04070	23.0			
POS-300 Z009 06 Z	20 11 20	31	Scamoline	neave started	43.64600	7 24 96 9	22.0	23.0	W3 20	
2000	10 10	Т		slock started	43.64600	7 24 837	30.6	Ī		
2002	20 12 02	1 92		at bottom	43.64602	7 24837	30.0			Boko 30
90	20 12 00	Т		heave started	43 64600	7 21833	32.5		W3 45	Box 33
2009 06	12	T	/ Observation platform	to water	43.64597	7.21893	20.8			
2009 06	12	Т		at depth	43.64602	7.21893	22.8		N3 18	
POS-386 2009 06 2	29 12 21	41	Seamonice/ Observation platform	at bottom	43.64608	7.21892	21.7		W3	
2009 06	29 12 22	2 42		at bottom	43.64612	7.21867	25.6	23.0	W3	WD max. 22m
2009 06	29 12 27	, 54		Station completed	43.64610	7.21850	28.8		N3	
2009 06	29 14 29	9 01 32-1	CPT	station arrived	43.64670	7.22100	14.9	23.3	W2	stations 376 cont.
2009 06	14	53		to water	43.64687	7.22102	15.6		W2	stations 376 cont.
90	29 14 55	2 10	CPT	in water	43.64682	7.22098	15.7		W2	Hydrophone t/water
2009 06	\neg	49		at bottom	43.64685	7.22100	16.0	23.4	N2 19	Hydrophone t/water - Boko 15m

40 40 4000	-				ſ		I		
POS-386 2009 06 30 12 49	03		on deck	43.64697	7.21927	25.8	24.6 V	WZ	
POS-386	22-5	Rn stations from Poseidon 3							
0000	Т			40.0440.0	T			9	
2009 06	20 24-3		at station	43.64/05	Ī	21.9		Z.M.	
06 30 13	П	Anchor for scuba divers	picked up	43.64710	7.21852	23.6		W2	
	40			43.64718		19.9		٧3	
POS-386 2009 06 30 13 39	39		at surface	43.64685	7.21757	32.7	24.6 V	W2	
POS-386 2009 06 30 13 40	29 37-4			43.64688		34.4	24.7 V	W2 5	
	48	CPT pilum		43.64698		34.1		72 35	Boko 31m
POS-386 2009 06 30 14 22	27	CPT pilum	heave started	43.64703	7.21780	32.8	22.5 V	W2 39	Boko 31m
POS-386 2009 06 30 14 24	20	CPT pilum	at surface	43.64705	7.21772	31.9	22.5 V	V2	
	23 37-5			43.64560		40.8		W2	station arrived
POS-386 2009 06 30 14 51	32	CPT pilum	at bottom	43.64560		41.1	22.7 V	W2 46	Boko 41 m
POS-386 2009 06 30 15 21	38		heave started	43.64563	7.21465	37.2	22.7 V	W2 48	
POS-386 2009 06 30 15 23	02		at surface	43.64562		41.9		٧2	
POS-386 2009 06 30 15 26	15	CPT pilum		43.64563	7.21478	41.1	22.8 V	V2	
POS-386 2009 07 01 06 09	18 38		station arrived	43.64142	7.21228	92.0	22.9 V	W4	
POS-386 2009 07 01 06 20	51	Piezometer		43.64260	7.21140	21.1	V 22.9	74	
POS-386 2009 07 01 06 28	29			43.64285	7.21135	14.7		W4	
POS-386 2009 07 01 07 15	14		new position arrived	43.64732		19.6		۷4	
POS-386 2009 07 01 07 24	02		to water	43.64752		12.9		W4	
POS-386 2009 07 01 07 26	37	Piezometer		43.64742		16.1	23.0 V	W4	Boko 16 m
POS-386 2009 07 01 07 31	13		water	43.64717	Γ	24.3		۸4	
POS-386 2009 07 01 07 33	42	Piezometer	station completed	43.64660		28.7		W4	
POS-386 2009 07 01 07 39	24 39	Gravity Corer	station started	43.64672		29.1		W3	
POS-386 2009 07 01 07 40	27		to water	43.64677		27.1		W3	
2009	41			43 64672	Ī	777		W3 38	Boko 30 m
2000	48		heave started	43 64672	7 21753	27.8			Boko 30 m
POS 386 2000 07 04 07 43	P 6			43.64677	Ť	7 80			DOWN 30 111
T	T		at surface	43.04077	Ī	20.7 28.5		W3	
2000 00 00 00 00 00 00 00 00 00 00 00 00	3 6			40.04670	T	20.0		2/4/2	I will find
POS-300 2009 07 01 07 40	30	39 (z. versul Gravity Corei		43.0467.0	Т	24.I		VV3	ZIN ulai
2009 07 01 07	Т		of hofform	43.64675	Γ	30.8		W3 40	Boko 30 m
07 01 07	Т	Gravity Corer	70	43.64675		30.6			Boko 32 m
Τ	95	Gravity Coros	CO.	43.04013 43.6467E	T	24.0		Ī	DONO OZ III
2000 07 01 07	Т	Gravity Corer	at surface	43.04073	T	3.5		WS	
POS-306 2009 07 01 07 05	0 40	Gravity Coros	off deck	43.04072	7.21737	33.3	73.1 V	W3	
Ϊ	T	Gravity Coles		40.04010	Τ	1.0		2 5	
POS-386 2009 07 01 08 07	10 40	Gravity Corer	nen	43.64515	7 21425	43.7	23.1 V	W3	
POS-386 2009 07 01 08 25	13		at bottom	43.64512		44.4		W3 63	Boko 55 m
POS-386 2009 07 01 08 25	18		ted	43.64513		44.7	23.1 V		Boko 55 m
POS-386 2009 07 01 08 27	03	Gravity Corer		43.64513		44.1	23.1 N	W3	
POS-386 2009 07 01 08 27	57			43.64517	7.21395	42.4	23.1 V	V3	
POS-386 2009 07 01 08 28	4	Gravity Corer	station completed	43.64518	7.21387	43.0	23.1 V	W3	
POS-386 2009 07 01 08 50	29 41	Gravity Corer	rived	43.64830		18.0		W3	
	26			43.64835		18.2	23.5 V	V3	
POS-386 2009 07 01 08 53	34	Gravity Corer	at bottom	43.64835	7.22568	18.3	23.5 V	W3 22	Boko 14 m
POS-386 2009 07 01 08 53	38	Gravity Corer	heave started	43.64835	7.22568	18.4	23.5 V	W3 22	Boko 14 m
	02			43.64838		18.6		٧3	
2009 07 01 08	╗			43.64835		19.0		W3	
POS-386 2009 07 01 08 56	80	orer	station completed	43.64835	7.22580	19.0	23.5 V	W3	
	46 42-1			43.63767		178.0			
T	13		to water	43.63767		180.0			- 1
07 01 10	Т	CPT cast	at bottom		T	174.0		W2 195	Boko 188m
POS-386 2009 07 01 11 04	18	CPI cast	heave started	43.63768	7.21118	1/0.0	23.5 V	V2 203	Boko 188m

43.63867 7.21090 141.0 23.6 43.63863 7.21090 146.0 23.8 43.63863 7.21090 146.0 23.8 43.63863 7.21080 146.0 23.8 43.63867 7.21088 108.0 23.7 43.63867 7.21088 108.0 23.7 43.63867 7.21088 118.0 23.9 43.63867 7.21088 118.0 23.9 43.63860 7.21085 111.0 23.9 43.63860 7.21085 111.0 23.9 43.63860 7.21085 111.0 23.9 43.63860 7.21085 111.0 23.9 43.63860 7.21085 111.0 23.9 43.64087 7.21107 112.0 23.9 43.64087 7.21107 64.0 24.2 43.64187 7.21108 13.4 24.2 43.64187 7.21108 13.5 24.1 43.64387 7.21108 13.5 24.1 43.64387 7.21108 13.1 24.1 43.64387 7.2120 20.9 24.2 43.64387 7.2120 20.9 24.2 43.64387 7.2120 20.9 24.2 43.64387 7.2120 20.9 24.2 43.64387 7.2120 20.9 24.2 43.64387 7.2120 20.9 24.2 43.64387 7.2120 20.9 24.2 43.64387 7.2120 20.9 24.2 43.64387 7.2128 31.0 24.2 43.64387 7.2128 31.0 24.2 43.64387 7.2128 31.0 24.2 43.64387 7.2128 20.8 43.64387 7.2128 31.0 24.2 43.6438 7.2128 31.0 24.2 43.6438 7.2128 31.0 24.2 43.6438 7.2128 31.0 24.2 43.6438 7.2128 31.0 24.3 43.6438 7.2128 31.0 24.3 43.6438 7.2128 31.0 23.3 43.6438 7.2128 31.0 23.3 43.6438 7.2128 31.0 23.3 43.6438 7.2128 31.0 23.3 43.6438 7.2128 31.0 23.3 43.6438 7.2128 31.0 23.3 43.6438 7.2128 81.0 23.3 43.6438 7.2128 31.0 23.3 43.6438 7.2128 31.0 23.3 43.6428 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 31.0 23.3 43.6438 7.2138 31.0 23.3 43.6438 7.2128 31.0 23.3 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 31.0 43.6438 7.2128 33.1 43.6438 7.2128 33.1 43.6438 7.2128 33.1 43.6438 7.2128 33.1 43.6438 7.2128 38.1 43.6438 7.2128 33.1 43.6438 7.2128 33.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.2128 38.1 43.6438 7.21108 31.0 43.6438 7.21108 31.0 43.6438 7.21108 31.0 43.6438 7.21108 31.0 43.6438 7.21108 31.0 43.6438 7.21108 31.0 43.6438 7.21108 31.0 43.6438 7.21108 31.0 43.6438 7.21108 31.0 43.6438 7.21	at surface	43.63768	7.21118	171.0	23.6	W2		
State	at surface	43.63867	7.21100	141.0	23.6	WZ		
Interest and the control of the co	to water	43.03870	7.21097	140.0	23.0		160	Bolo 151m
Exercise	at Dottolli heave started	43.63857	7.21080	146.0	23.8		168	Boko 151m
Exercise	atsurface	43.63855	7.21080	145.0	23.8			
Sinck stated 43,58967 27,008 108	atsurface	43.63967	7.21088	109.0	23.7	W2		
Standard	to water	43.63967	7.21088	108.0	23.7	W2	2	
Heavy String	slack started	43.63962	7.21095	117.0	23.7	W2	2	
Heave started 43,52960 17,000 11,00 23.9 W2 12,10 11,00 11,00 11,0	at bottom	43.63968	7.21085	110.0	23.7		121	Boko 116m
Internal content	heave started	43.63960	7.21093	117.0	23.9		121	Boko 116m
Internal continue	at surface	43.63960	7.21095	113.0	23.9	W2	121	Boko 116m
Standard	to water	43.63960	7.21093	113.0	23.9	W2		
Stank started 43,64687 72,170 71,20 23.9 W2 5	at surface	43.64087	7.21125	110.0	23.9	W2		
Reversified	slack started	43.64087	7.21107	112.0	23.9		2	
Itemses started	atbottom	43.64095	7.21092	101.0	23.8		108	Boko 102m
Statistical	heave started	43.64093	7.21090	0.86	24.0		111	Boko 102m
to moteck to make the stand to the completed to the compl	atsurface	43.64082	7.21085	100.0	24.0	W2		
Interior	on deck	43.64082	7.21085	0.86	24.0	W2		
Beautified 1,24,14 1,24,15 1	to water	43.64193	7.21107	64.0	24.2	W2		
Base started	at bottom	43.64187	7.21125	62.0	24.1		9/	Boko 72m
Standard	heave started	43.64177	7.21120	0.59	24.0	W2	77	
at surface	at surface	43.64167	7.21117	0.69	24.1	W2	77	
In owater	atsurface	43.64383	7.21063	13.4	24.2	W2		
alt bottom	to water	43.64383	7.21063	13.4	24.2	W2		
Holewe started	at bottom	43.64387	7.21083	13.5	24.2		19	Boko 14m
Standard	heave started	43.64390	7.21090	13.5	24.1		25	Boko 14m
Station arrived	atsurface	43.64388	7.21080	13.3	24.1	W2		
Table Tabl	at surface	43.64523	7.21207	20.9	24.2			
All control of the	to water	43.64518	7.21203	21.9	24.2			
Station completed	at bottom	43.64510	7.21208	31.1	24.1		56	Boko 22m
Assumance	neave started	43.64507	7.21213	33.8	24.0		39	Boko 22m
rements driggly rements rements driggly rements rements driggly rements rement	at surface	43.64508	122127	38.8	24.0	WZ		
Station completed	on deck	43.04448	7.04000	0.1.0	24.0	ZWZ		
Station arrived	station completed	43.64437	0.2712.7	35.6	23.9	WZ		
Station arrived 43.64708 7.21657 23.6 23.3 W4								
Station arrived 43.64708 7.21667 23.6 23.3 W4								
Benthos on deck 43.647/8 7.21665 21.5 23.3 W4 heave started 43.647/2 7.21672 21.8 23.3 W4 out of bottom 43.647/2 7.21672 21.8 23.3 W4 Plezometer at surface 43.647/3 7.217/3 19.4 23.3 W4 heav position arrived 43.642/8 7.227/8 36.5 23.2 W4 to water 48.643/8 7.227/8 36.5 23.2 W4 at bottom 43.643/8 7.227/8 36.5 23.2 W4 station completed 43.643/8 7.227/9 38.5 23.2 W4 at surface 43.642/8 7.227/9 14.3 23.3 W3 at surface 43.642/8 7.2109 14.3 23.3 W3 at surface 43.642/8 7.2109 14.1 23.3 W3 at surface 43.642/8 7.2110 14.0 23.3 W3 at surface 43.642/8 7.2110 44.0 23.3 W3 at surface 43.642/8 7.2110 23.3 W3 at surface 43.642/8 7.2110 44.0 23.3 W3 at surface 43.642/8 7.2110 44.0 23.3 W3 at surf	station arrived	43.64708	7.21657	23.6	23.3	W4		
heave started 43.64722 7.21672 21.8 23.3 W4 out of bottom 43.64732 7.21673 19.4 23.3 W4 Plezometer at surface 43.64732 7.21633 19.4 23.3 W4 new position arrived 43.64282 7.22278 36.5 23.2 W4 to water 43.64328 7.22278 36.5 23.2 W4 at bottom 43.64328 7.22278 36.1 23.2 W4 station completed 43.64320 7.22278 36.1 23.2 W4 station arrived 43.6432 7.2229 38.5 23.3 W3 station arrived 43.64277 7.21090 14.3 23.3 W3 station arrived 43.64278 7.21090 14.1 23.3 W3 station arrived 43.64278 7.2100 14.1 23.3 W3 station arrived 43.64285 7.21100 14.1 23.3 W3 station arrived 43.64285 7.21100 14.1 23.3 W3 station arrived 43.64285 7.21108 14.0 23.3 W3 station arrived 43.64280 7.21108 14.0 23.3 W3 station arrived 43.64280 7.21108 14.0 23.3 W3 station arrived 43.64280 7.21108 14.0 23.3 W3 station arrived 7.21080 7.21080 7.21080 7.21080 7.21080 7.2	Benthos on deck	43.64718	7.21665	21.5	23.3	W4		
out of bottom 43.64732 7.21693 19.4 23.3 W4 Plezometer at surface 43.64733 7.21713 17.7 23.3 W4 Inew position arrived 43.64328 7.22278 61.0 23.2 W4 Inew position arrived 43.64328 7.22278 86.1 23.2 W4 Inew position arrived 43.64328 7.22277 36.1 23.2 W4 Benthos to water 43.64332 7.22277 37.4 23.2 W4 Inextension completed 43.64332 7.22277 37.4 23.2 W4 Inextension completed 43.64332 7.22177 37.4 23.2 W4 Inextension completed 43.64332 7.22109 14.3 23.3 W3 Inextension completed 43.64232 7.2109 14.3 23.3 W3 In the completed 43.64278 7.21100 14.1 23.3 W3 23 In the completed 43.64278 7.21100 14.1 23.3	heave started	43.64722	7.21672	21.8	23.3	W4		
Plezometer at surface 43.64733 7.21713 11.7 23.3 W4 Inew position arrived 43.64282 7.22328 61.0 23.2 W4 It has been been been been been been been bee	out of bottom	43.64732	7.21693	19.4	23.3	W4		
New position arrived 43.64282 7.22328 61.0 23.2 W4 Ito water	Piezometer at surface	43.64733	7.21713	17.7	23.3	W4		
to water 45.64228 7.2278 36.5 23.2 W4 at the control of the contro	new position arrived	43.64282	7.22328	61.0	23.2	W4		
1 bottom 43.64332 7.2273 36.1 23.2 W4	to water	43.64328	7.22278	36.5	23.2	W4		
Benthos to water 43,6430 7,2277 37,4 23.2 W4	at bottom	43.64332	7.22273	36.1	23.2	W4		Boko 36 m
Station completed 43.6432 7.2292 38.5 23.2 W4	Benthos to water	43.64330	7.22277	37.4	23.2	W4		
Station arrived 43.64277 7.21090 14.3 23.3 W3	station completed	43.64332	7.22292	38.5	23.2	W4		
Items	station arrived	43.64277	7.21090	14.3	23.3	W3		
at bottom 43.64278 7.21100 14.3 23.3 W3 23.3 m3 23.3 m	to water	43.64282	7.21095	14.2	23.3			!
neave started	at bottom	43.64278	7.21100	14.3	23.3		23	Boko 15 m
at suffice	neave started	43.64278	7.21100	14.1	23.3		73	Boko 15 m
	at sulface	43.04263	7 21118	14.0	23.3	W3		
S S S S S S S S S S S S S S S S S S S	OII deck	49.04293	7.044.00	14.0	60.0	CW.		
ravity corer		at surface at surface to water at bottom heave started at surface to water slack started at surface to water slack started at surface to water at surface to water at surface on deck to water at surface at surface at surface at surface on deck to water at surface at surface at surface at surface at surface at surface on deck to water at bottom heave started at surface on deck heave started at surface to water station completed station arrived Benthos to water station arrived beave started at surface at surface to water station onpleted to water station onpleted station arrived station arrived to water at bottom heave started at surface no water station arrived station arrived station arrived at surface on deck at surface on deck		45.63867 45.63867 45.63867 45.63867 45.63867 45.63867 45.63867 45.63867 45.63867 45.63867 45.63867 45.63867 45.63867 45.64087 45.64087 45.64087 45.64087 45.64087 45.64087 45.64087 45.64087 45.64087 45.64087 45.64087 45.64087 45.6438 45.6438 45.6437 45.6437 45.6437 45.6437 45.6432 45.6432 45.6432 45.6432 45.6432 45.6432 45.6432 45.6433 45.6428 45.64283 45.64283 45.64283 45.64283 45.64283	43.6367 43.6367 43.63867 43.63863 43.63863 43.63864 43.63867 43.64387 43.64288 43.64388 43.64288	43.63676 43.63676 43.63867 7.2100 41.03867 43.63867 7.2100 43.63867 7.2100 43.63867 7.2100 44.00 43.63867 7.2100 44.00 43.63867 7.2100 44.00 43.63867 7.2100 43.64087 7.2108 41.00 43.64087 7.2108 41.00 43.64087 7.2108 41.00 43.64087 7.21108 43.6418 7.21107 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6418 7.21108 43.6432 7.21108 43.6432 7.21108 43.6432 7.21108 43.6432 7.21108 43.6432 7.21108 43.6432 7.21108 43.6432 7.21108 43.6432 7.21108 43.6432 7.21108 43.6432 7.21108 43.64332 7.21108 43.64332 7.21108 43.64332 7.21108 43.64332 7.21108 43.64332 7.21108 43.64282 7.21108 43.64282 7.21108 43.64282 7.21108 43.64282 7.21108 43.64282 7.21108 43.64282 7.21108 43.64283 7.21118 44.0 44.0 44.0 44.0 44.0 44.0 44.0 44.	45.83670 7.2109 1410 23.6 WZ 45.8367 7.2109 1410 23.6 WZ 45.8363 7.2109 146.0 23.6 WZ 45.8365 7.2109 146.0 23.8 WZ 45.8367 7.2109 146.0 23.8 WZ 45.8367 7.2109 146.0 23.7 WZ 45.8367 7.2108 118.0 23.7 WZ 45.8368 7.2108 118.0 23.9 WZ 45.8360 7.2108 113.0 23.9 WZ 45.84087 7.2108 113.0 23.9 WZ 45.84087 7.2108 113.0 23.9 WZ 45.84087 7.2109 113.4 24.2 WZ 45.84193 7.2105 113.4 Z4.2 WZ 45.84193 7.2105 113.4 Z4.2 WZ 45.84193 7.2106 113.5 Z4.1 WZ 45.8419 7.2107 20.9 24.2 WZ 45.8419 7.2108 113.6 Z4.1 WZ 45.8419 7.2107 20.9 24.2 WZ 45.8419 7.2108 113.6 Z4.1 WZ 45.8420 7.2108 113.6 Z3.9 WZ 45.8430 7.2108 113.6 Z3.9 WZ 45.8430 7.2108 113.6 Z3.9 WZ 45.8430 7.2128 31.0 Z3.3 WZ 45.8430 7.2128 31.0 Z3.2 WZ 45.8430 7.2128 31.0 Z3.2 WZ 45.8432 7.2227 35.6 Z3.3 WZ 45.8432 7.2227 35.6 Z3.3 WZ 45.8427 7.2228 36.5 Z3.2 WZ 45.8427 7.2227 33.4 Z3.3 WZ 45.8427 7.2227 33.4 Z3.3 WZ 45.8427 7.2227 33.4 Z3.3 WZ 45.8427 7.2228 36.5 Z3.3 WZ 45.8428 7.22108 14.0 Z	45.83670 7.2109 1410 23.6 WZ 45.8367 7.2109 1410 23.6 WZ 45.8363 7.2109 146.0 23.6 WZ 45.8365 7.2109 146.0 23.8 WZ 45.8367 7.2109 146.0 23.8 WZ 45.8367 7.2109 146.0 23.7 WZ 45.8367 7.2108 118.0 23.7 WZ 45.8368 7.2108 118.0 23.9 WZ 45.8360 7.2108 113.0 23.9 WZ 45.84087 7.2108 113.0 23.9 WZ 45.84087 7.2108 113.0 23.9 WZ 45.84087 7.2109 113.4 24.2 WZ 45.84193 7.2105 113.4 Z4.2 WZ 45.84193 7.2105 113.4 Z4.2 WZ 45.84193 7.2106 113.5 Z4.1 WZ 45.8419 7.2107 20.9 24.2 WZ 45.8419 7.2108 113.6 Z4.1 WZ 45.8419 7.2107 20.9 24.2 WZ 45.8419 7.2108 113.6 Z4.1 WZ 45.8420 7.2108 113.6 Z3.9 WZ 45.8430 7.2108 113.6 Z3.9 WZ 45.8430 7.2108 113.6 Z3.9 WZ 45.8430 7.2128 31.0 Z3.3 WZ 45.8430 7.2128 31.0 Z3.2 WZ 45.8430 7.2128 31.0 Z3.2 WZ 45.8432 7.2227 35.6 Z3.3 WZ 45.8432 7.2227 35.6 Z3.3 WZ 45.8427 7.2228 36.5 Z3.2 WZ 45.8427 7.2227 33.4 Z3.3 WZ 45.8427 7.2227 33.4 Z3.3 WZ 45.8427 7.2227 33.4 Z3.3 WZ 45.8427 7.2228 36.5 Z3.3 WZ 45.8428 7.22108 14.0 Z

2009 07 02 07 48		station arrived	43.63877	7.21373	133.0		W3		
2009 07 02 07		to water	43.63882	7.21363	131.0				
07 02 07 52		at bottom	43.63898	7.21375	123.0				Boko 154 m
2009 07 02 07 52		heave started	43.63898	7.21375	121.0		W3 162		Boko 154 m
POS-386 2009 07 02 07 55 56	Gravity corer	at surface	43.63902	7.21390	120.0	23.2 V	W3		
	Gravity corer	on deck	43.63902	7.21390	119.0	23.2 V	V3		
POS-386 2009 07 02 07 57 43	Gravity corer	station completed	43.63897	7.21387	126.0	23.2 V	W3		
POS-386 2009 07 02 08 19 50 48	Gravity corer	station arrived	43.63010	7.21360	382.0		W3		
	Gravity corer	to water	43.63002	7.21357	388.0		V3		
2009 07		at bottom	43.62998	7.21363	392.0				BoKo 428m
П		heave started	43.63000	7.21363	404.0		W3 436		
POS-386 2009 07 02 08 39 24		on deck	43.63002	7.21240	388.0				
POS-386 2009 07 02 08 52 55	Gravity corer	station arrived	43.62978	7.21370	388.0	23.6 V	W3		
POS-386 2009 07 02 08 59 03	Gravity corer	to water	43.63005	7.21357	383.0	23.7 V	W3		
	Gravity corer	at bottom	43.62998	7.21353	392.0				Boko 437m
POS-386 2009 07 02 09 04 03	Gravity corer	heave started	43.62997	7.21353	388.0	23.7 V	V3 445		Boko 437m
POS-386 2009 07 02 09 11 57	Gravity corer	on deck	43.63023	7.21328	381.0	23.6 V	W3		
POS-386 2009 07 02 09 12 53	rer	Station completed	43.63028	7.21323	380.0	23.6 V	V3		
		Station arrived	43.64205	7.21100	0.66	24.0 V	W2 5		
POS-386 2009 07 02 11 31 26	CPT cast	to water	43.64203	7.21102	0.66	24.0 V	W2 5		
POS-386 2009 07 02 11 40 16		slack started	43.64197	7.21102	0.66				
POS-386 2009 07 02 11 41 23	CPT cast	at bottom	43.64207	7.21098	0.66	24.2 V	W2 67	Bok	Boko 62m
POS-386 2009 07 02 12 17 53		heave started	43.64207	7.21103	25.0	24.1 V	V2 67	Bok	Boko 62m
POS-386 2009 07 02 12 20 00		at surface	43.64215	7.21105	44.2	24.0 V	W2		
POS-386 2009 07 02 12 23 50		on deck	43.64233	7.21095	38.1	23.9 V	W2		
POS-386 2009 07 02 12 49 22 49-2		to water	43.64393	7.21090	13.4	24.0 V	W2		
POS-386 2009 07 02 13 06 46		slack started	43.64398	7.21088	13.6		W2 5		
	CPT cast	at bottom	43.64403	7.21087	13.3	23.8 V	V2 23	Bok	Boko 18m
		heave started	43.64403	7.21087	13.6	24.0 V	W2 28	Bok	Boko 18m
-		at surface	43.64407	7.21092	13.3		V2		
2009 07 02 13 43 12	CPT cast	on deck	43.64412	7.21097	13.3		W2		
T		to water	43.64535	7.21192	15.0	24.2 V	W2		
,	CPT cast	slack started	43.64537	7.21193	14.4		W2 5		
		at bottom	43.64537	7.21190	12.9				
POS-386 2009 07 02 13 58 53		at bottom	43.64538	7.21188	12.7	24.1 V	W2 29	Bok	Boko 25m
2009 07 02 14 30	CPT cast	heave started	43.64552	7.21197	18.5		W2 46	Bok	Boko 25m
T	CPT cast	at surface	43.64550	7.21217	25.5		W2		
2009 07 02 14 47 47		at surface	43.64567	7.21330	41.3				
POS-386 2009 07 02 14 49 16 49-4	CPT cast	to water	43.64570	7.21337	42.2				
1		slack started	43.64573	7.21328	43.7				
1	CPT cast	at bottom	43.64585	7.21327	35.0			Bok	Boko 39m
POS-386 2009 07 02 15 25 51	CPT cast	heave started	43.64565	7.21340	41.4		W2 47	Bokc	Boko 39m
Ì	CPT cast	at surface	43.64570	7.21335	38.9		W2		
2009 07 02 15 55 45		at surface	43.64677	7.21923	24.3				
POS-386 2009 07 02 15 55 50 49-5	CPT cast	to water	43.64678	7.21923	25.9		W2 5		
2009 07 02 16 01		slack started	43.64682	7.21927	25.0				
POS-386 2009 07 02 16 01 59		at bottom	43.64685	7.21925	24.6		W2 31	Bok	Boko 25m
07 02 16 31	CPT cast	heave started	43.64678	7.21913	24.8	24.0 V	W2 35	Bokc	Boko 25m
		at surface	43.64695	7.21913	26.5		V2		
2009 07 02 16 44 17	CPT cast	at surface	43.64660	7.21975	18.0				
Т	CPI cast	to water	43.64660	7.21983	17.8		WZ 5		
2009 07 02 16 49	CPI cast	slack started	43.64673	7.21987	18.9	24.1		200	Dolog 4 Ones
2000 07 00 00 00 00 00 00 00 00 00 00 00	UPI Gast	at bottom	43.04072	7.21982	10.1			BOK	1.40m
POS-386 Z009 07 0Z 17 Z3 57	CPI cast	neave started	43.64673	7 24075	19.5	24.0	WZ 31	POK	Boko 19m
2009 01 02 11 23	OF L Cast	atsullace	10.040.0	01612.1	20.0		٧٧		

	Towns.			ſ	Ī	Ī		
2009 07 02 17 56 36		at surface	43.64/08	1				
02 17 57 15	49-7 CPT cast	to water	43.64708				2	
П	CPT cast	slack started	43.64723	1			2	
2009 07 02 18 04	CPT cast	at bottom	43.64720			23.8 W2	19	Boko 14m
POS-386 2009 07 02 18 31 56	CPT cast	heave started	43.64723	7.22255	12.4	24.1 W2	19	Boko 14m
	CPT cast	at surface	43.64728			24.1 W2		
POS-386 2009 07 02 18 39 06	CPT cast	on deck, station completed	43.64630	7.22293	14.6	24.1 W2		
POS-386 50								
2009 07 03 05	Piezometer	station arrived	43.64245					
2009 07 03 06 06	Piezometer	Top buoy picked up	43.64305					
POS-386 2009 07 03 06 10 10	Piezometer	heave started	43.64300		53.0	23.7 W4		
	Piezometer	on deck	43.64312					piezometer pile bended - download data
POS-386 2009 07 03 06 44 06	Piezometer	station completed	43.63668		0			piezometer pile bended - download data
POS-386 2009 07 03 07 05 03 51	1 ec	station arrived	43.64382	7.21870	15.5	23.8 W3		
POS-386 2009 07 03 07 06 13	29	to water	43.64383					
POS-386 2009 07 03 07 09 36	29	at bottom	43.64402	7.21913	14.9	23.8 W3	20	Boko 12 m
POS-386 2009 07 03 07 09 50	29	heave started	43.64402	7.21910	14.8	23.8 W3	20	Boko 12 m
POS-386 2009 07 03 07 13 17		atsurface	43.64395	7.21917		23.9 W3		
POS-386 2009 07 03 07 14 19		on deck	43.64395			23.9 W3		
POS-386 2009 07 03 07 14 43		station completed	43.64393	7.21917	15.5	23.9 W3		
POS-386 2009 07 03 07 32 18 52	29	station arrived	43.64385	7.22227		23.9 W3		
POS-386 2009 07 03 07 51 19		to water	43.64378			23.8 W3		
POS-386 2009 07 03 07 52 36	29	at bottom	43.64380	7.22225	25.9	23.8 W3	29	Boko 21m
POS-386 2009 07 03 07 52 39	29	heave started	43.64380	7.22225	26.0	23.8 W3	29	Boko 21m
POS-386 2009 07 03 07 54 10	29	atsurface	43.64380	7.22230	25.7	23.8 W3		
POS-386 2009 07 03 07 55 24		on deck	43.64378	7.22223	24.5	23.8 W3		
POS-386 2009 07 03 07 55 44		station completed	43.64385			23.8 W3		
POS-386 2009 07 03 08 23 08 53	29	station arrived	43.64750			23.9 W3		
POS-386 2009 07 03 08 26 14		to water	43.64752	7.21678	16.7	24.0 W3		
POS-386 2009 07 03 08 27 47	29	at bottom	43.64750				22	Boko 14 m
POS-386 2009 07 03 08 27 51	29	heave started	43.64750	7.21677	18.3	24.0 W3	22	Boko 14 m
POS-386 2009 07 03 08 28 57	29	at surface	43.64752	7.21677	17.6	24.0 W3		
POS-386 2009 07 03 08 29 55	29	on deck	43.64753			24.0 W3		
POS-386 2009 07 03 08 30 11	29	station completed	43.64753	7.21677	16.9	24.0 W3		
2009 07 03 10 01 52	54-1 Piezometer CPT	station arrived	43.64292	7.21068	14.0	24.0 W2		
POS-386 2009 07 03 10 06 54	Piezometer CPT	to water	43.64302	T				
T	Piezometer CPT	slack started	43.64303	7.21072				
2009 07 03 10	Piezometer CPT	slack started	43.64302	7.21063			2	
POS-386 2009 07 03 10 18 07	Piezometer CP1	at bottom	43.64307	7.21063			50	Boko 16m
T	Piezometer CP1	heave started	43.64313	7.21073	T		26	Boko 16m
2009 07 03 10 28 40		ат ѕитасе	43.64305	1			97	Boko 16m
07 03 10 49 20	54-2 Plezometer CP1	to water	43.64/43	7.21668	Ī		Ωι	
Ť	Plezometer CP1	slack started	43.64/42	1	Ī		מי	
2009 07 03 10 51	Plezometer CP1	slack stopped	43.647.50	T			2	2
07 03 10 51	Plezometer CP1	at bottom	43.04/4/	7.21662	7.0.7	24.4 WZ	٥,7	B0N0 22M
Ť	Plezometer CP I	neave started	45.04742					
2009 07 03 10 53 23		at Surface	43.64/40	T				
07 03 11 10 27	54-3 Plezometer CP1 cast	station started	43.04333	7 99967	45.7	24.4 WZ	L	
2009 07 03 11 10	Piezometer CP1 cast	to water	43.64333				Ω	
2009 07 03 11 19	Plezometer CP1 cast	slack started	43.64337	T			2	
POS-386 2009 07 03 11 19 44	Plezometer CP1 cast	at bottom	43.64337	7.22245	39.2	24.2 WZ	40	Boko 38m
2009 07 03 11 21	Plezometer CPT cast	heave started	43.64330	T			40	Boko 36m
2009 07 03 11 23	Plezometer CP1 cast	at surface	43.04330	T				
POS-386 2009 07 03 11 37 43 POS-386 2009 07 03 11 38 16 F2	FALA Diszometer CPT cast	at sunace to water	43.04388	7 21957	16.7	24.4 VVZ	Ľ	
2003 07 03 11 30 10		to water	43.04300	1			2	

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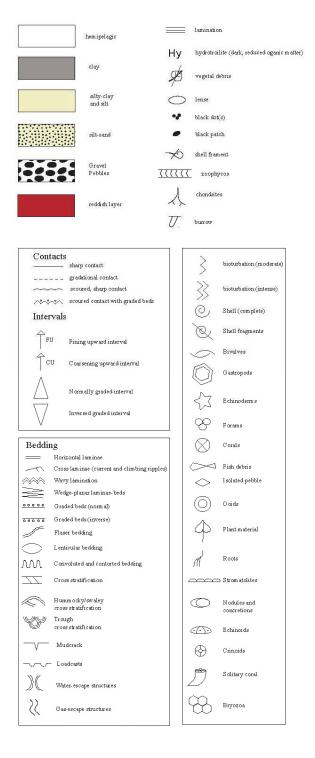
24.9	7.22300 7.2	Ш
7.2	7.2230	
7.3	7.22	
8.9	7.22	
7.22298 6.9 24.9	72	
6.9	7.22	
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21.4	7.22	43.64538 7
21.9	7.22	_
20 22.0 25.0	7.22420	43.64535 7.224 43.64533 7.224
22.6	7.224	
18.6	7.22165	Station started, at surface 43.64422 7.221
72 188 24.8	7 221	43.64425 7.22.100
18.6	7.22163	Ĺ
18.4	7.22165	Ħ
18.3	7.221	
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7.22163 18.5 24.8	2	43.64430 7.22
263 33.4 24.2	7 21263	43 64422
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42.4	7.21	
34.8	7.21	7
7.21807 69.0 24.4	7.21	43.64240 7.21
71.0	7.	
72.0	7.2	
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7.21783 44.2 24.4	7.2	43.64313 7.2
44.3	7.21	
47.0	7.21	
24.5	7.21	43.64320 7.21
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68.0	\sim 1	43.64198
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7 21105 13.3 25.0		43.04382

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	07 04	12	28 26	9		ре							Boko 14m
	07 04	12	34 54	4					13.4	25.3 V	W2 40)	
	07 04	12	П	8 60-3	CPT				30.8	25.5 V	W2 5		
П	07 04	12 [59 15	5				П	34.9	25.1 V	W2		
2009	07 04	13	00 32	2			43.64597	7.21398	31.7	25.2 V	N2 36		BoKo 31m
POS-386 2009	07 04	13	П	3					32.3	25.1 V		6	
POS-386 2009	07 04	13	32 03	П				П	31.5	25.1 V	W2		
POS-386 2009	07 04	13	T	6 60-4	CPT		43.64558		49.0		W2 5		
2009	07 04	13 4	42 23	3					50.0		N2 5		
2009	07 04	13	43 09						49.7				Boko 40m
2009	07 04	14	11 51				43.64560	7.21320	43.1		W2 50		Boko 40m
	07 04	14	14 10	0 CPT					49.7		N2		
	07 04	14	39 15			/ at surface			24.3	25.5 V	W2		
2009	07 04	14	39 46			to water	43.64740	7.21672	24.9	25.4 V	W2 5		
2009	07 04	14 4	41 26			pq			24.9		N2 5		
POS-386 2009	07 04	14 4	42 16						24.6	25.4 V	N2 5		on bootom laying
POS-386 2009	07 04	14 4	42 42			pei			24.0	25.4 V	W2 5		
POS-386 2009	07 04	14 4	42 57						25.0	25.4 V	N2 5		
POS-386 2009	07 04	14 4	43 36						24.1	25.4 V	N2 5		
POS-386 2009	07 04	14 4	44 01	1				7.21663	23.8	25.4 V	N2 5		
POS-386 2009	07 04	14 4	45 04				43.64755			25.4 V	W2 22		Boko 18m
	07 04	14 4	47 06								N2 22		Boko 18m
2009	07 04	14	48 05						22.9	25.3 V	W2		
POS-386 2009	07 04	14 5	54 45						27.9		W2		
	07 04	14	55 14						29.1	25.5 V	W2 5		
2009	07 04	14	56 25					7.21663	28.8	25.5 V	W2 5		
POS-386 2009	07 04	14	57 05	2			43.64703				N2 35		Boko 28m
	07 04	15 ,	26 40	0							N2 38		Boko 28m
	07 04	15	36 00								W2		
		16 0	08 48	9-09 8			43.64323	7.22265			N2		
	07	16	09 33	3		to water			46.0				
T	07 04	16	12 00	0									
-	07 04	16	12 47	7									Boko 44m
T	07 04	19	31 39	6			43.64332		38.3		W2 55		Boko 44m
	07 04	16					43.64335				W2		
POS-386 2009	07 04	16	54 15	2 60-7		Station arrived / at surface	43.64680	T					
T	07 04	16	55 OC				43.64680	1	36.3				
T	07 04	16	56 27		CPT			T		25.5 V	W2 5		مدارد مارد
POS-386 2009	07 04	2 5	26 27			heave started	43.04032	7 22813	33.7				Boko 39mi
POS-386 2009	07 04	<u> </u>	30 42	2			Г	Τ	35.3				
Г	07 04	17 4	40 33	3			43.64655		44.9		N2		
POS-386 2009	07 04	17	41 07	8-09 2	CPT	to water	43.64657	7.22907	44.8		W2 5		
POS-386 2009	07 04	17 4	41 59	6		slack started	43.64655	7.22913	45.7	25.2 V	W2 5		
	07 04	17 4	43 01	1	CPT		43.64662		45.6	25.2 V	W2 53		Boko 47m
	07 04	. 81	11 08	8					44.6	25.3 V	N2 58		Boko 47m
2009	07 04	18 1	12 45			at surface	43.64663	7.22907	44.3	25.3 V	W2		
2009	07 04	18 1	18 12		CPT	on deck / station completed	43.64640	7.22908	47.8	25.2 V	W2		
			T		n Var mouth with dinghy								
2009	05	90									W3		
2009	6	90	15 16				43.64087	7.21808	84.0	24.5 V	W3		
POS-386 2009	02	90	Т			at bottom/heave started						124	Boko 116m
5000	CO L	90	Т								W3		
POS-386 2009	07 05 06	1	39 33	1	97 20	station arrived	43.64395	7 24553	74.0	24.5	W3		
2002			1								20		

Boko 92m					Boko 15 m	Boko 15 m			2nd Trial	Boko 18 m	Boko 18 m			3rd trial	Boko 14 m	Boko 14 m			Research works completed
101					23	23				56	56				22	22			
W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3	W3
24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.6	24.6	24.6	24.6	24.6	24.7
75.0	74.0	73.0	13.1	13.4	15.5	15.0	21.2	20.8	19.5	18.8	18.7	16.9	16.4	19.2	19.6	19.7	20.3	20.8	20.4
7.21545	7.21553	7.21553	7.21678	7.21677	7.21670	7.21670	7.21658	7.21652	7.21650	7.21663	7.21663	7.21665	7.21662	7.21678	7.21682	7.21682	7.21682	7.21675	7.21672
43.64408	43.64413	43.64413	43.64750	43.64757	43.64755	43.64755	43.64747	43.64743	43.64747	43.64747	43.64747	43.64750	43.64753	43.64750	43.64748	43.64748	43.64745	43.64745	43.64745
at bottom	on deck	on deck	station arrived	to water	at bottom	heave started	atsurface	on deck	to water	at bottom	heave started	atsurface	on deck	to water	at bottom	heave started	at surface	on deck	station completed
9 GC	39 80	3 60	3 64 GC	2 GC	0 90	4 GC	25 6	1 60	1 64-1 GC	29 0	29 9	25 8	1 60	1 64-2 GC	25 0	4 GC	1 60	4 GC	2 GC
46 05	49 38	49 53	08 03	18 52	19 40	19 44	50 59	22 11	24 01	25 20	25 26	26 48	28 01	38 11	08 68	39 34	41 21	45 04	42 42
90	90 9	90		20 9	2 07	20 90		20		20	20	20	20	2 07	2 07	20	20		07
07 05	02 05	02 02	07 05	02 0	07 05	20	90 20	02 02	90 20	02 02	90 20	02 02	90 20	02 02	02 02	02 02	90 20	90 20	02 0
2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009	2009
POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386	POS-386

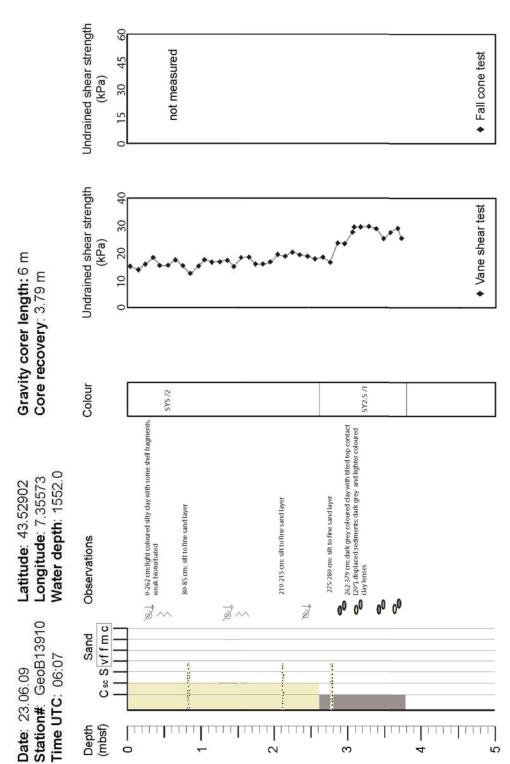
10.2 Lithologs and shear strength data

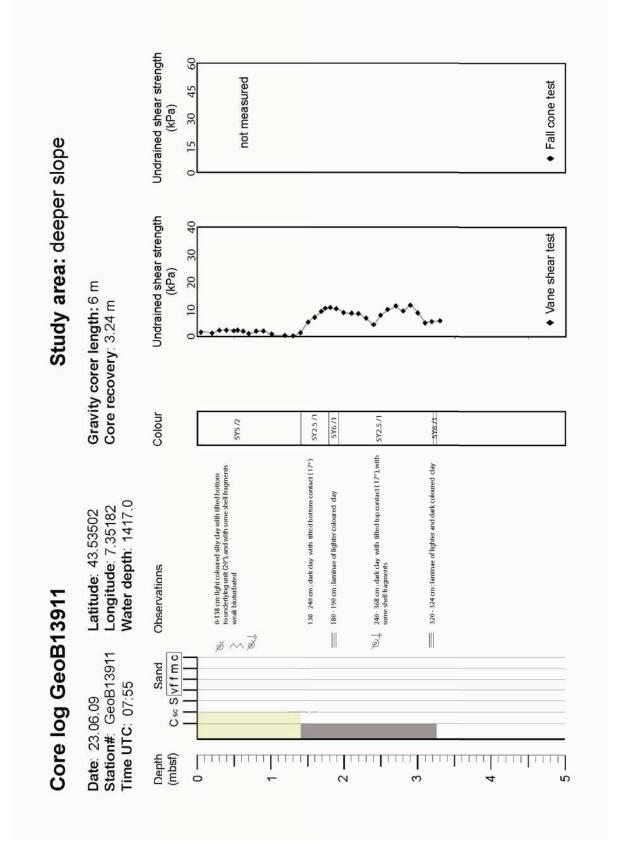
Legend:



Core log GeoB13910

Study area: deeper slope





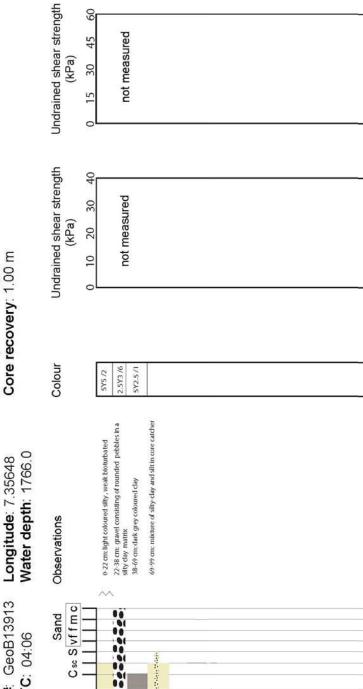
Study area: deeper slope

Gravity corer length: 6 m

Longitude: 7.35648 Latitude: 43.51085 Date: 24.06.09 Station#: GeoB13913 Time UTC: 04:06

Depth (mbsf)

0



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not measured

30

15

- 130 -

Fall cone test

Vane shear test



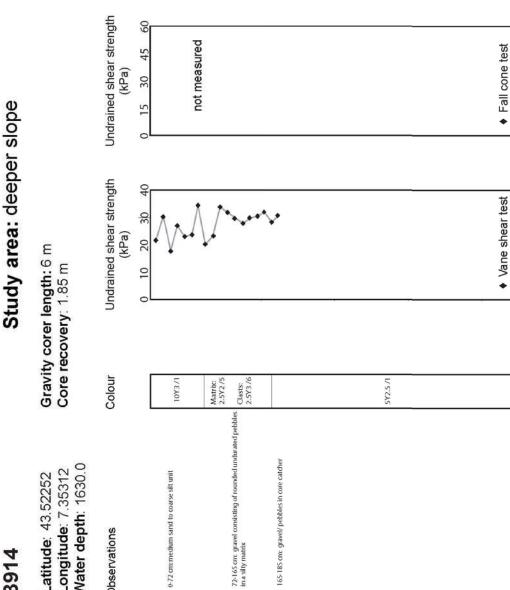
Water depth: 1630.0 Latitude: 43.52252 Longitude: 7.35312 Date: 24.06.09
Station#: GeoB13914
Time UTC: 05:58

Observations

Csc S vffmc Sand

Depth (mbsf)

0



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Date: 27.06.09 **Station#**: GeoB13918 Time UTC: 08:33

Latitude: 43.62770

Longitude: 7.21407

Water depth: 397 m

Gravity corer length: 6m

Study area: Nice airport

Core recovery: 3 m

Undrained shear strength

Colour

Observations

Csc S vf f m c

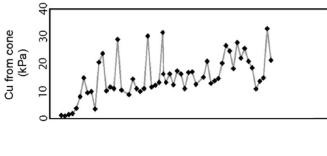
Depth (mbsf)

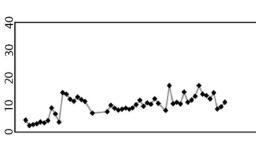
0

Sand

5 Y4/1 5 Y3/2

0 - 2cm: Foam 2 - 11,5cm: weak clay, water rich 11,5 - 13cm: silty clay, water rich





3 /10Y

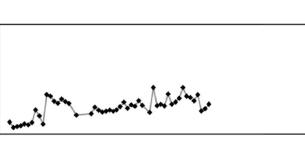
150 - 152 cm: silty lense 155 - 160 cm: black laminae, organic stuff 171 - 172, 175-177 cm: silty lenses

133 - 134 cm: light gray colour, clay

98- 100cm: Foam 108-109: light gray colour, clay 110, 118-120 cm : black laminia

2.5 /10Y 6 /10Y

3 10GY



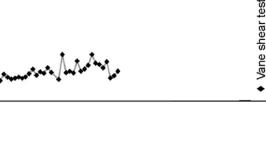
573 /2

prodominate clay 226 - 330 cm: darker colour but also clay

191, 196 cm : black laminae 196 - 198 cm: silty lenses 200 - 300 cm: clay to silty clay

7

SV 2.5 /2



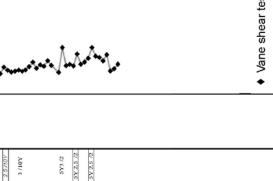
290-300 cm: Gravel Curved quartzite and tonstein

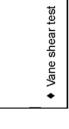
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275 - 276 cm: silt to fine sand lense







Fall cone test

- 132 -

Core log GeoB13919

Date: 27.06.09

Longitude: 7.22248 E Latitude: 43.64317 N Station#: GeoB13919

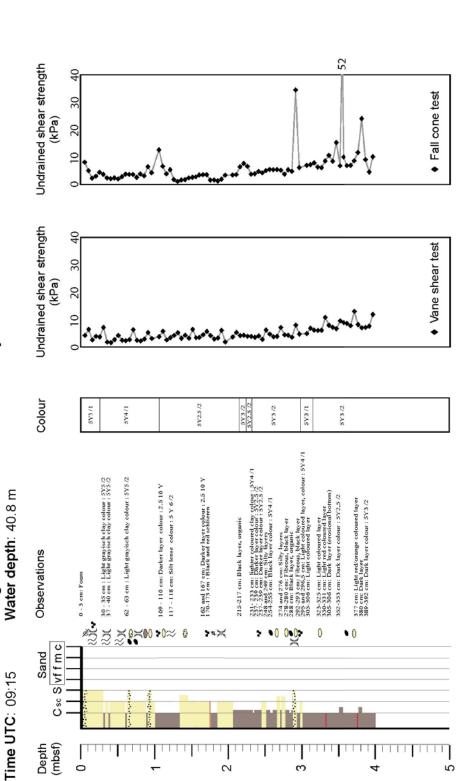
Water depth: 40.8 m

Depth (mbsf)

0



Gravity corer length: 6m Core recovery: 4 m



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Core log GeoB13925

Study area: Nice airport

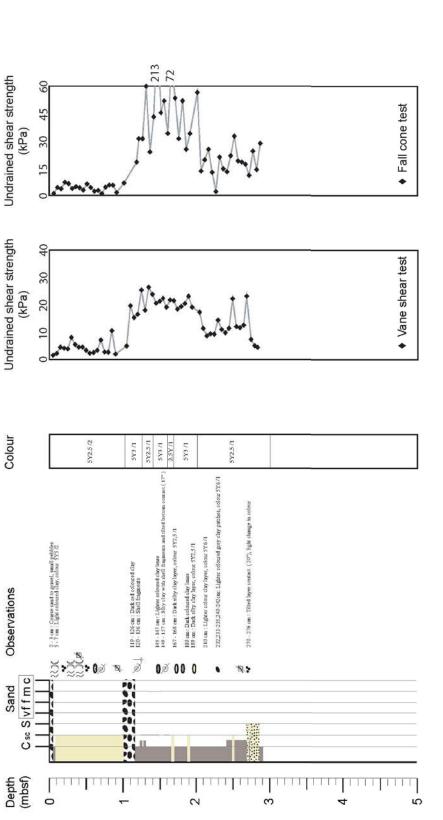
Gravity corer length: 6 m Core recovery: 2.90 m

Latitude: 43.64600 Longitude: 7.21853

Date: 28.06.09 Station#: GeoB13925

Time UTC: 06:42

Water depth: 30.1



Study area: Nice airport

Station#: GeoB13926 Time UTC: 08:18 Date: 28.06.09

Longitude: 7,22368 Latitude: 43.64507

Water depth: 19.8



Colour

Observations

Csc S vffmc Sand

Depth (mbsf)

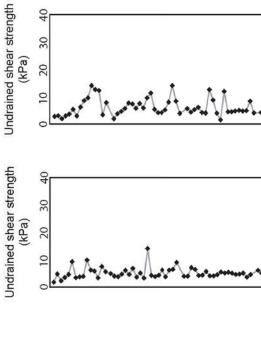
2.57471

17 - 19 cm : Light coloured clay, colour : 2.5YS /1

0 05

0

Gravity corer length: 6 m



2.5YS /2

81,5 - 82 cm : Reddish clay, colour : 2.5YR4 /4 83,5 - 84,5 cm : Light coloured clay, colour : 2.5YR4 /4

65 - 67 cm : Light coloured clay, colour : 2.5Y6 /2

573 /2

31ey 13/10

143 - 146,5 cm : Light coloured clay, colour : 5Y5 /1

162 cm : Black patch, organic

109, 112 cm; Reddish patches 109-111; cm; Black pinth dain 123 - 126 cm; Light colourac (lay, colour; 5373/11 129,5 - 130,5 cm; Darker clay, colour; 532.5/1

5Y4/1

209 - 213 cm : Water rich silty clay, colour : 5Y6 /1

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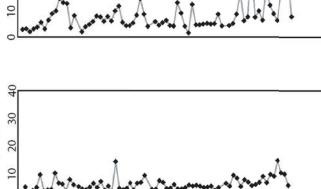
257 - 258 cm : Light coloured clay, colour : 5Y7 /1

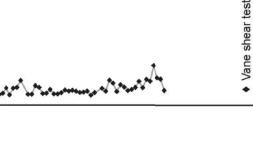
Section 1

276 cm : Reddish layer

234 cm : clay, colour : 5Y8 /3 235 cm : Reddish clay, colour : 2.5YR4 /3

30 20





301 cm; Redditch brown layer, colour 12.5578.4.14
312.5 cm; Light coloured day, colour 1578.7.1
312.5 cm; Light coloured day, colour 1578.7.1
314.5 115 cm; Gradel bedding, downwards
314.525 cm; Light coloured day, colour; 2.577.11
326.527 cm; Light coloured day, colour; 2.577.11

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349-351 cm : Darker silty-clay, colour : 2,5Y3 /1 361-362 cm : Dark clay, colour : 5Y2 5 /1

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5Y5/1	2.5Y6/1	2.574/1



Fall cone test

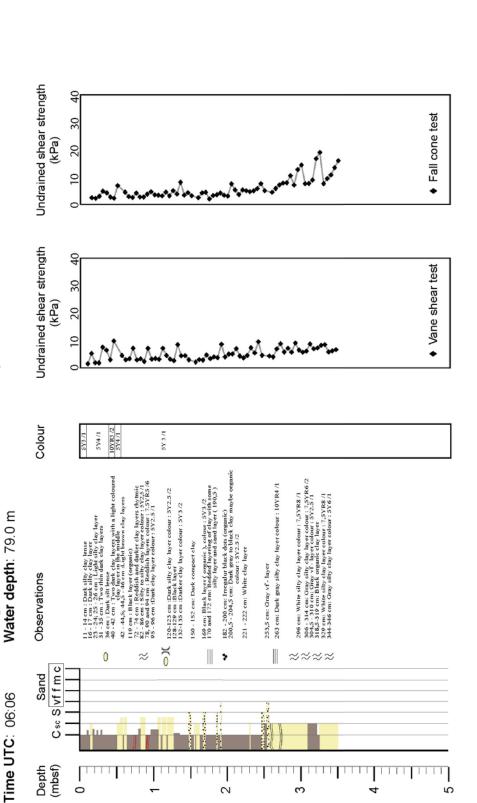
Date: 29.06.09 Latitude: 43.64133 N

Longitude: 7.21332 E

Station#: GeoB13928

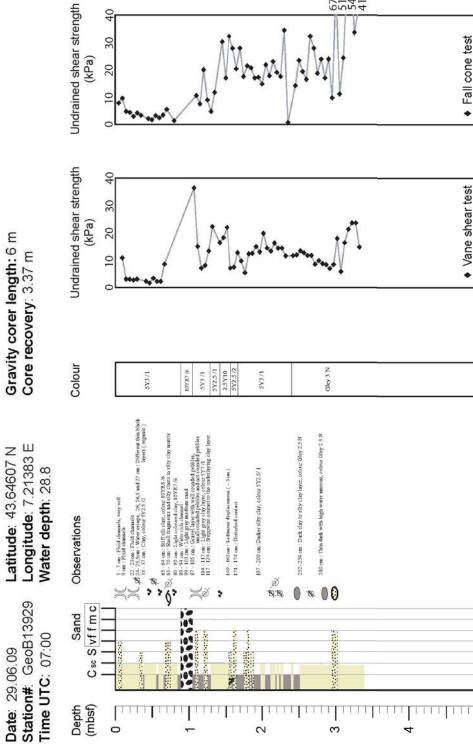
Gravity corer length: 6 m Core recovery: 3.51 m

Study area: Nice airport



Core log GeoB13929

Study area: Nice airport



30

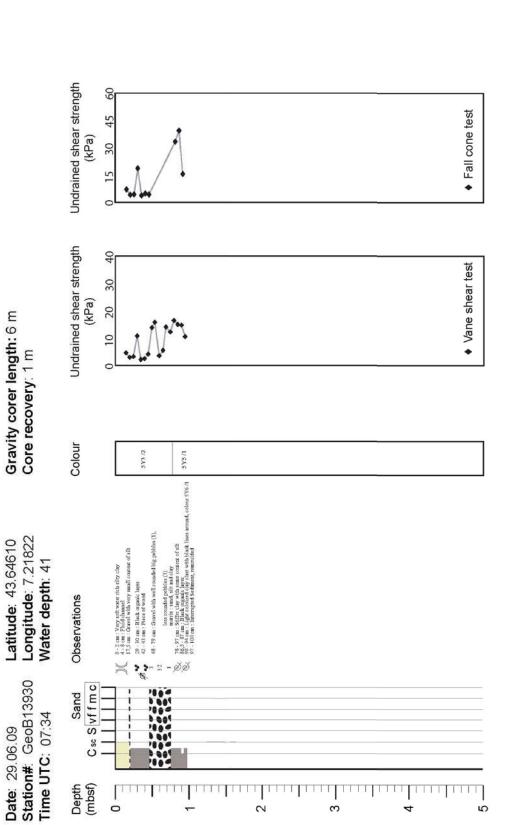
67,6 51,96 54,93 41,18

Core log GeoB13930

Study area: Nice airport

Gravity corer length: 6 m

Latitude: 43.64610



Date: 30.06.09 Station#: GeoB13934 Time UTC: 07:13

Depth (mbsf)

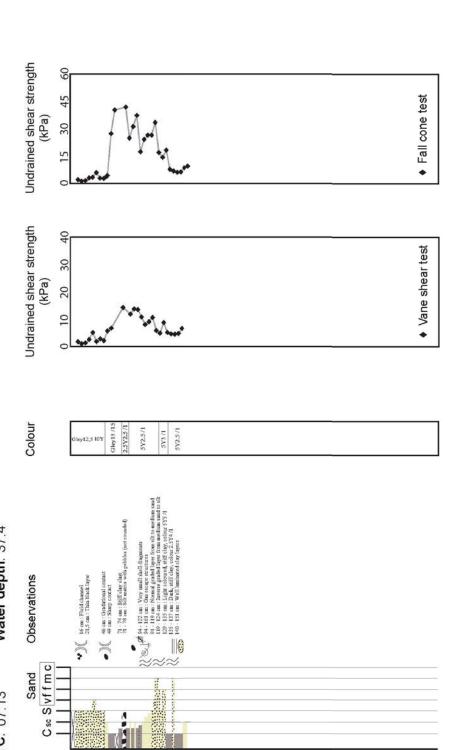
0

Latitude: 43.64562 N

Longitude: 7.21315 E Water depth: 37.4

Study area: Nice airport

Gravity corer length: 6 m Core recovery: 1.61 m



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Station#: GeoB13939 Time UTC: 07:41 Date: 01.07.09

Longitude: 7.21755 Latitude: 43.64672

Water depth: 27.7

Observations

Csc S vffmc Sand

Depth (mbsf)

0

0.1 cm; Watercitish

1.415 cm; Black patishen, colour 577.71

1.10 cm; This bodded filly flayers, dark to light

2.6 - 67 cm; Black don

48 cm : Adhesive tape

144 cm: Water rich silty/clay layer, colour 5Y7 /1 155 - 160 cm: Bodded layering of dark layers

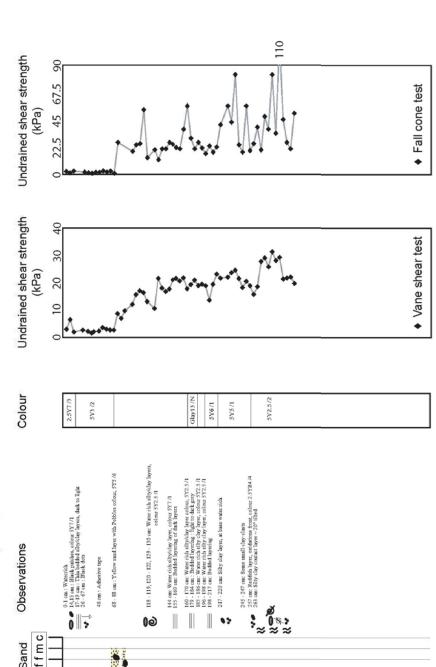
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(6)

Study area: Nice airport

Gravity corer length: 6 m Core recovery: 3.19 m



Station#: GeoB13940 Time UTC: 08:25 Date: 01.07.09

Latitude: 43.64512

Water depth: 44.4 m Longitude: 7.21413

Observations

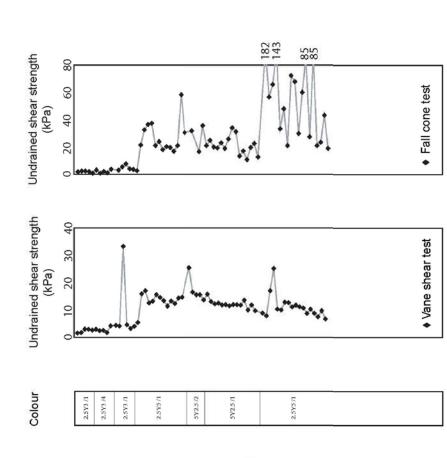
Csc S vffmc Sand

Depth (mbsf)

0

Gravity corer length: 6 m Core recovery: 3.52 m

Study area: Nice airport



O & 172 - 174 cm : Hard clay with villense, colour, 5Y3 /1

7

(2) 150 cm : Black patch

211 cm : Lighter colour clay leave, colour, 5Y6 /1.

223 cm : Water rich siky clay layer 233 - 248 cm : Higher water centent 192,5 - 195 cm : Water rich leuse

汉 📗 250 - 340 cm : Gas escape siru

76, 78, 79 cm : Light gray clay layer, colour, 5Y7 /1

80 - 83 cm : Water rich vf layer, colour, 2,5Y5 /2

340 cm : White patch (clay - stity clay), colour, 5Y8/1

3

Date: 02.07.09 **Station#**: GeoB13946 **Time UTC**: 07:24

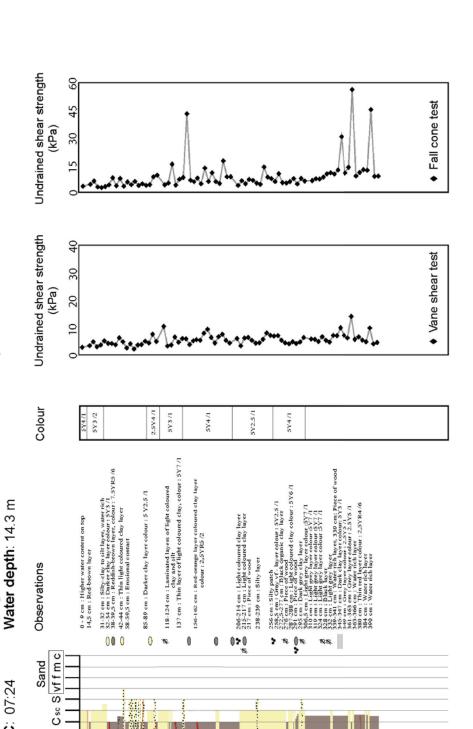
Depth (mbsf)

0

Longitude: 7.21100 E Latitude: 43.64278 N



Gravity corer length: 6 m Core recovery: 4.09 m



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Study area: Nice airport

Station#: GeoB13952 Time UTC: 07:52 **Date**: 03.07.09

Longitude: 7.22225 Latitude: 43.64380

Observations

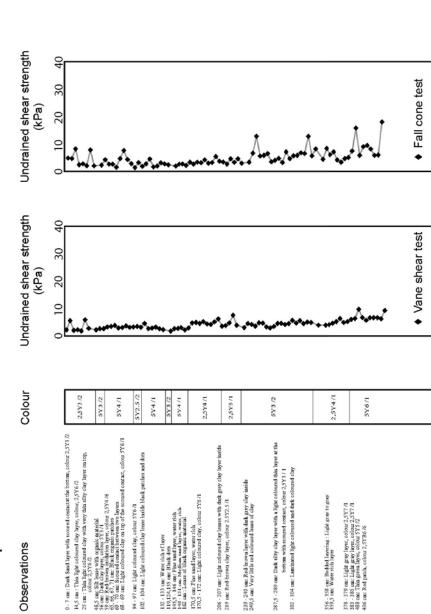
Csc S vffmc Sand

Depth (mbsf)

0

Water depth: 25.9

Gravity corer length: 6 m Core recovery: 4.42 m



170,5 cm: Fine sand layer, water rich 170,5 - 172 cm: Light coloured clay, colour 5Y6/1

1122 - 133 cm: Water rich v Flayer 119, 124, 135 cm: Black dots 135, 146 cm: Plan sand layer, water rich 145 - 151 cm: Medium sand layer, water rich 162 cm: Lease of black organis material

2

1117531175311

356 - 358 cm: Bedded layering : Light gray to gray 359,5 cm: Water rich layer

378 - 379 cm: Light gray layer, colour 2,5V7 /1 383 - 385 cm: Light gray layer, colour 2,5V7 /1 488 cm: Thin green layer, colour 2,5Y7 /1 406 cm: Rod parth, colour 5.Y5 /6

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Core log GeoB13953

Date: 03.07.09 **Station#**: GeoB13953 Time UTC: 08:27

Sand

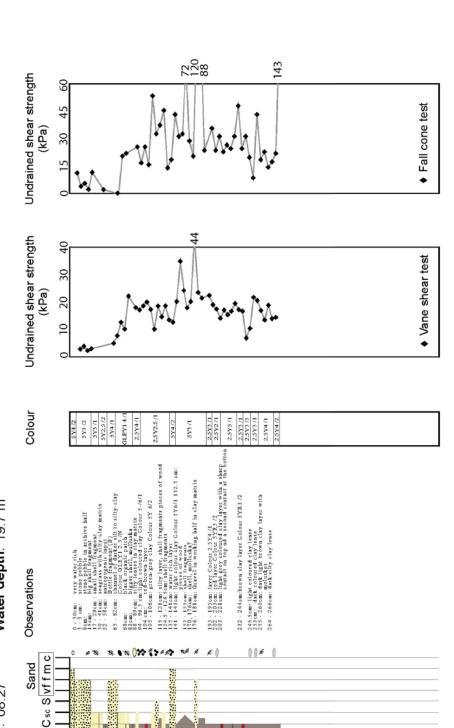
Depth (mbsf)

0

Water depth: 19.7 m Longitude: 7.21677 Latitude: 43.64750

Study area: Nice airport

Gravity corer length: 6 m Core recovery: 2.85 m



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10.3 MSCL data logs and core photographs (available in electronic version for cruise participants only)

Index of CD-ROM in back pocket (for cruise participants only):

- PDF file of cruise report
- Appendix 10.1 as XLS file
- Appendix 10.2 as PDF files
- Appendix 10.3 as PDF files
- Tables 3 through 6 (Geochemistry) as DOC files

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No. 1 Wefer, G., E. Suess and cruise participants

Bericht über die POLARSTERN-Fahrt ANT IV/2, Rio de Janeiro - Punta Arenas, 6.11. - 1.12.1985. 60 pages, Bremen, 1986.

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Holozänstratigraphie und Küstenlinienverlagerung an der andalusischen Mittelmeerküste. 173 pages, Bremen, 1988. (out of print)

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Stabile Kohlenstoff-Isotope in partikulärer organischer Substanz aus dem Südpolarmeer (Atlantischer Sektor). 161 pages, Bremen, 1989.

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1. Kolloquium des Sonderforschungsbereichs 261 der Universität Bremen (14.Juni 1991): Der Südatlantik im Spätquartär: Rekonstruktion von Stoffhaushalt und Stromsystemen. Kurzfassungen der Vorträge und Poster. 66 pages, Bremen, 1991.

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Berichtszeitraum Oktober 1990 - Dezember 1992. 396 pages, Bremen, 1992.

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